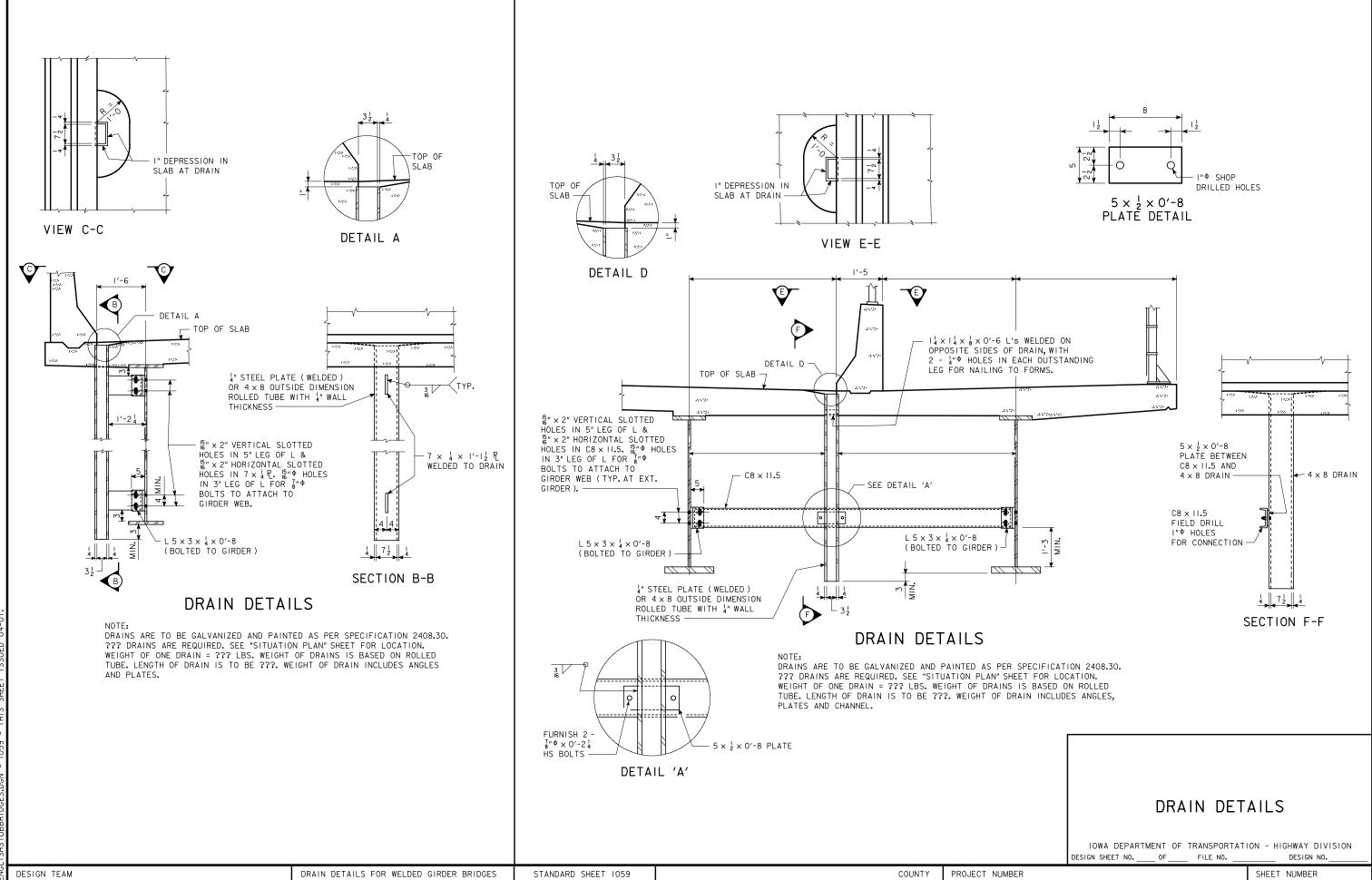
	INDEX OF STUB BRIDGE STANDARDS
STANDARD	DESCRIPTION
2092	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - O SKEW
2093	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 0°01-7°30 SKEWS
2094	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 7°31-15 SKEWS
2095	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 15°01-30 SKEWS
2096	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 0°01-7°30 SKEWS
2097	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 7°31-15 SKEWS
2098	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 15°01-30 SKEWS
2099	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - O SKEW
2100	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 0°01-7°30 SKEWS
2101	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 7°31-15 SKEWS
2102	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (R.A.) 15°01-30 SKEWS
2103	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 0°01-7°30 SKEWS
2104	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 7°31-15 SKEWS
2105	"C" OR "D" BEAMS - STUB ABUTMENT DETAILS - (L.A.) 15°01-30 SKEWS
4305	30' WELDED CROSS SECTION LRFD DESIGN
4305A	ALTERNATE INTERMEDIATE DIAPHRAGM FOR WELDED GIRDER BRIDGES
4308	40' WELDED CROSS SECTION LRFD DESIGN
4309	44' WELDED CROSS SECTION LRFD DESIGN
4310	40' WELDED CROSS SECTION (SYMM CROWN) LRFD DESIGN
4542	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT., O SKEW
4543	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (L.A.) 0°01 - 7°30 SKEW
4544	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (L.A.) 7°31 - 15° SKEW
4545	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (L.A.) 15°01 - 30° SKEW
4546	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (R.A.) 0°01 - 7°30 SKEW
4547	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (R.A.) 7°31 - 15° SKEW
4548	PART PLAN & LONGIT. SECT "B", "C", & "D" BEAMS, STUB ABUT. (R.A.) 15°01 - 30° SKEW
4549	STUB ABUT. "B", "C", & "D" BEAMS, BAR LIST & SUPER. DETAILS - O SKEW
4550	STUB ABUT. "B", "C", & "D" BEAMS, BAR LIST & SUPER. DETAILS - 0°01 - 7°30 SKEW
4551	STUB ABUT. "B", "C", & "D" BEAMS, BAR LIST & SUPER. DETAILS - 7°31 - 15° SKEW
4552	STUB ABUT. "B", "C", & "D" BEAMS, BAR LIST & SUPER. DETAILS - 15°01 - 30° SKEW
4556	30' RDW. PPCB ("B", "C", & "D" BEAMS - STUB ABUT.) CROSS SECTION
4559	40' RDW. PPCB ("B", "C", & "D" BEAMS - STUB ABUT.) CROSS SECTION
4560	44' RDW. PPCB ("B", "C", & "D" BEAMS - STUB ABUT.) CROSS SECTION
4561	40' RDW. PPCB ("B", "C", & "D" BEAMS - STUB ABUT.) CROSS SECTION (SYMM CROWN)

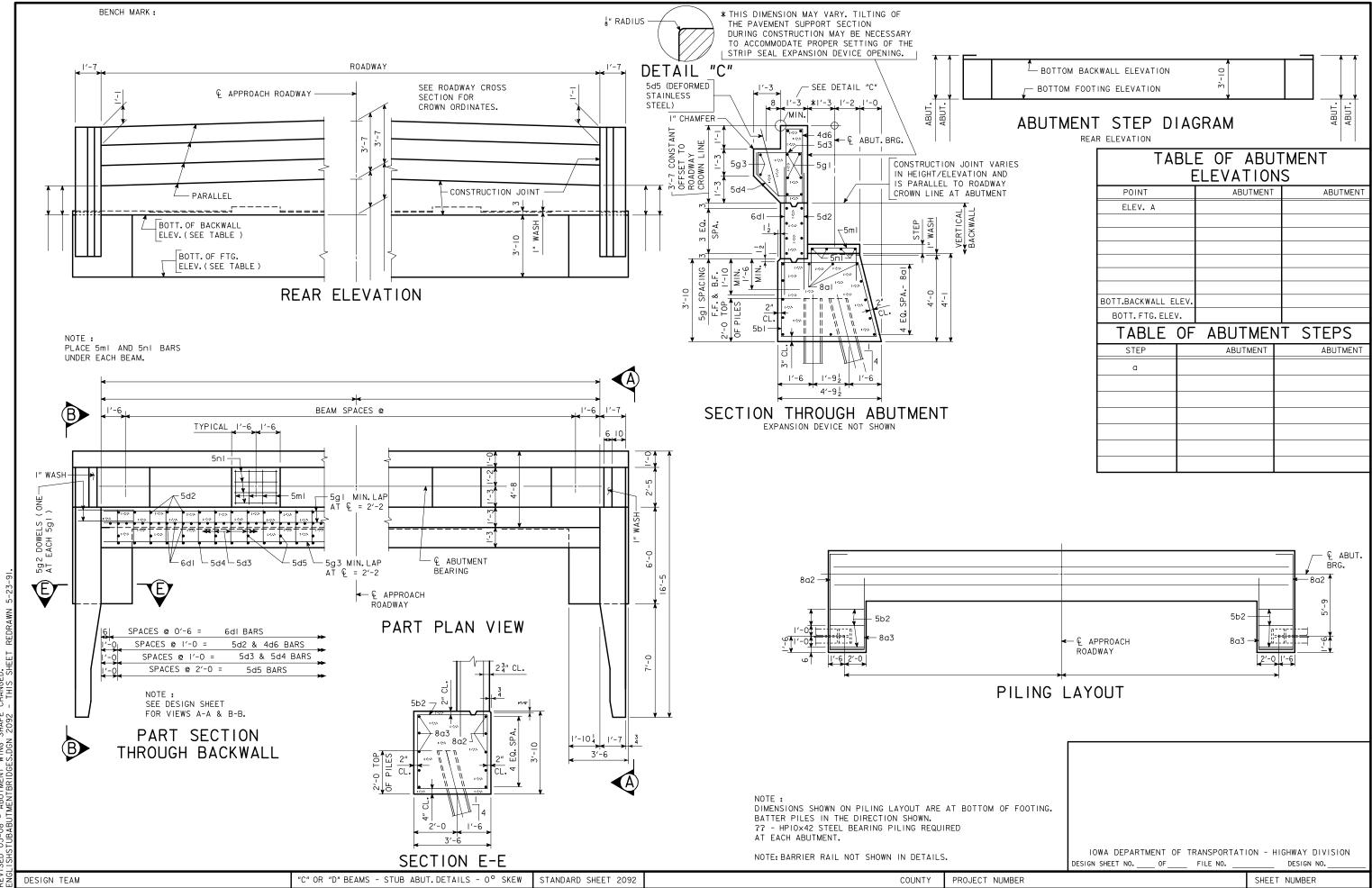
INDEX OF STUB BRIDGE STANDARDS

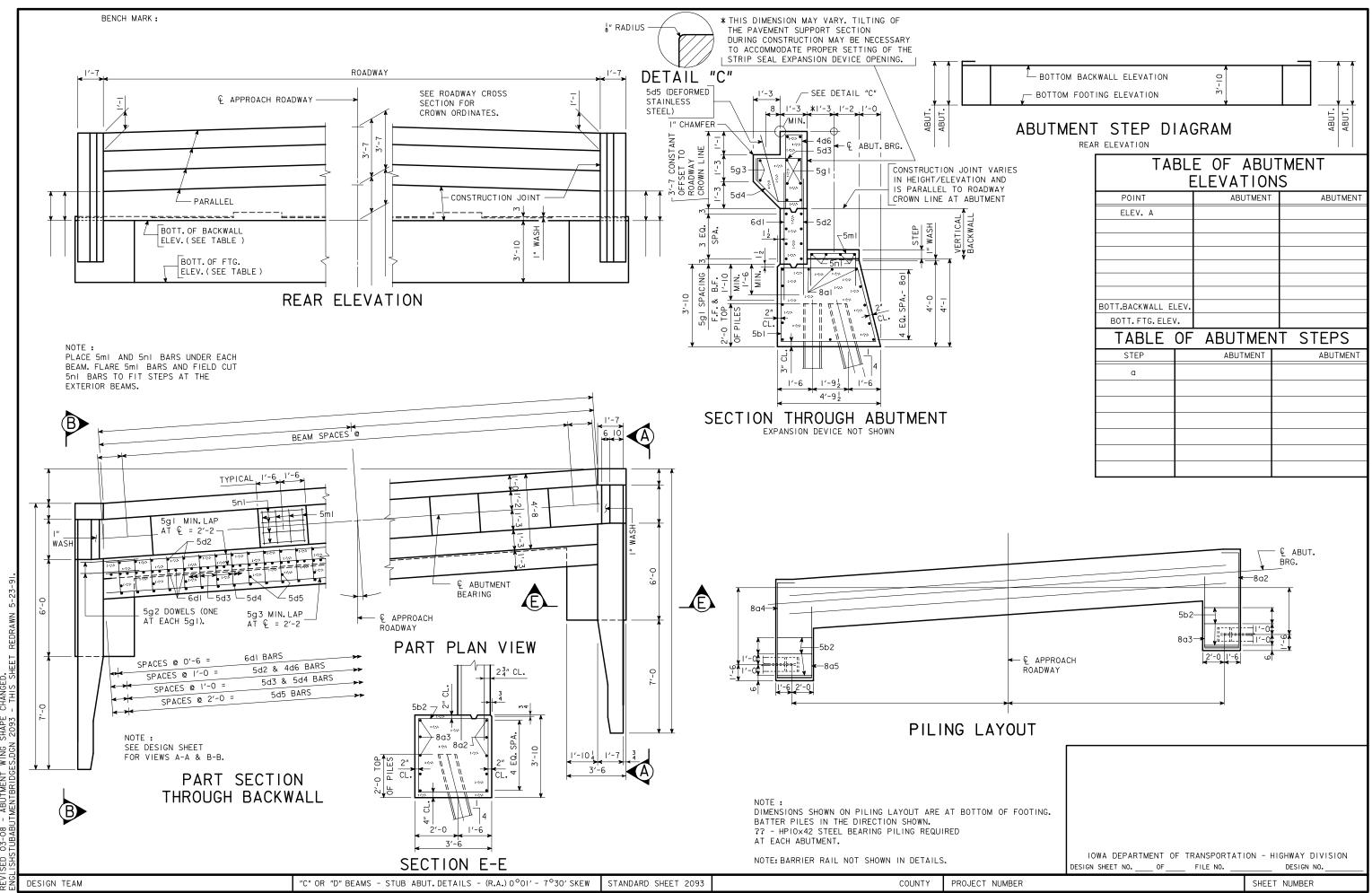
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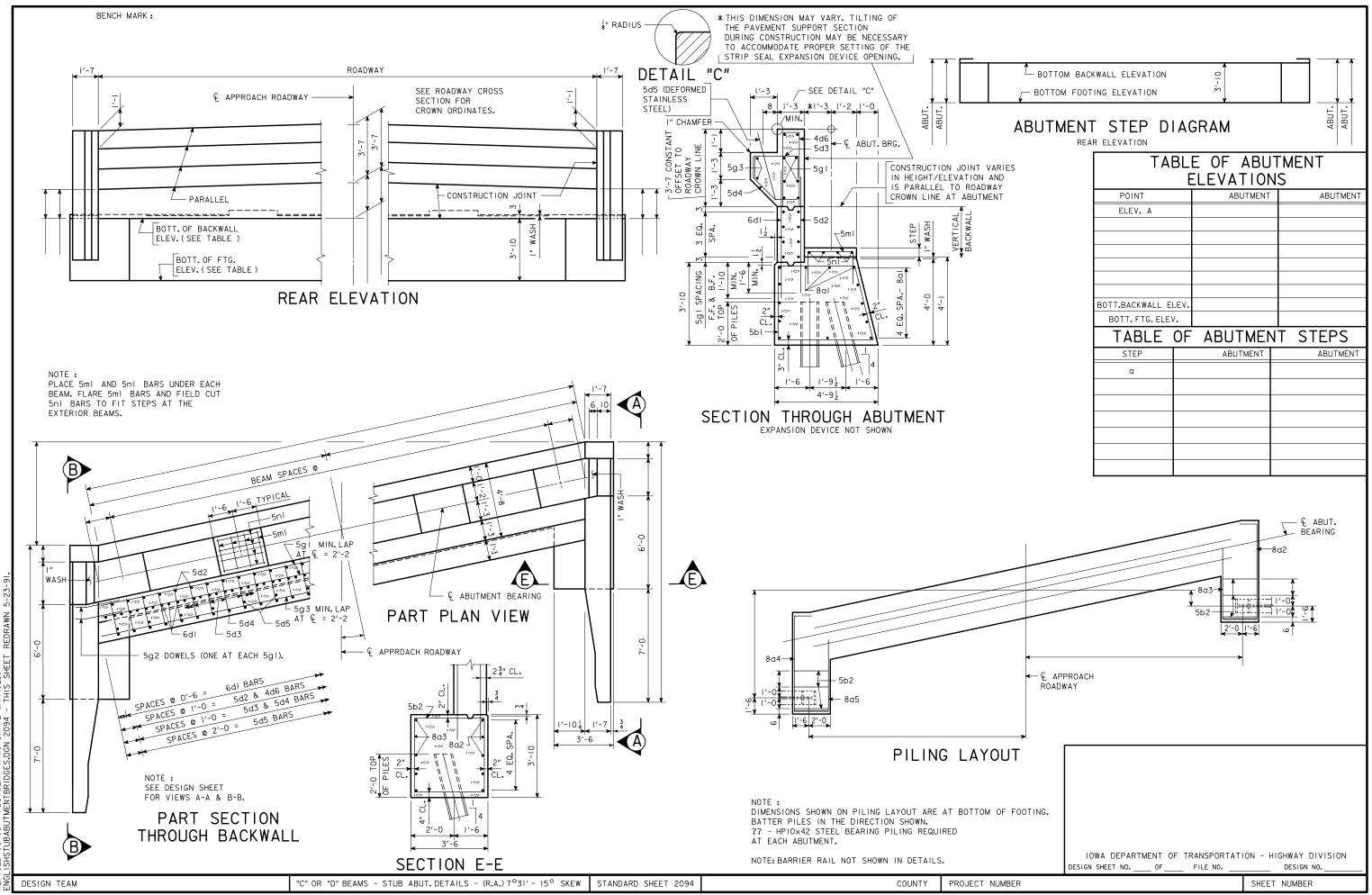
INDEX OF STUB ABUTMENT BRIDGE STANDARDS STANDARD SHEET 100-S COUNTY PROJECT NUMBER SHEET NUMBER

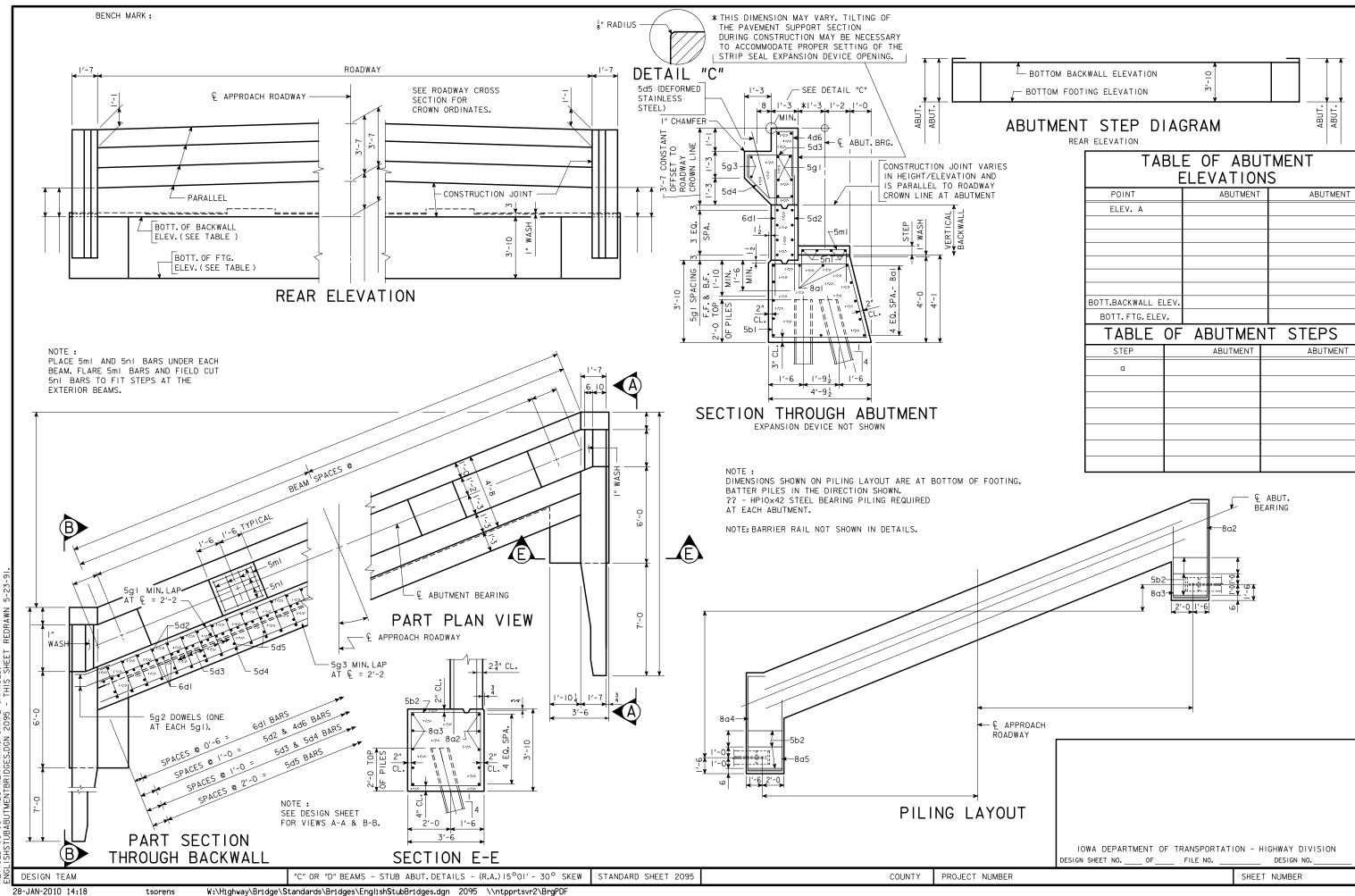
tsorens

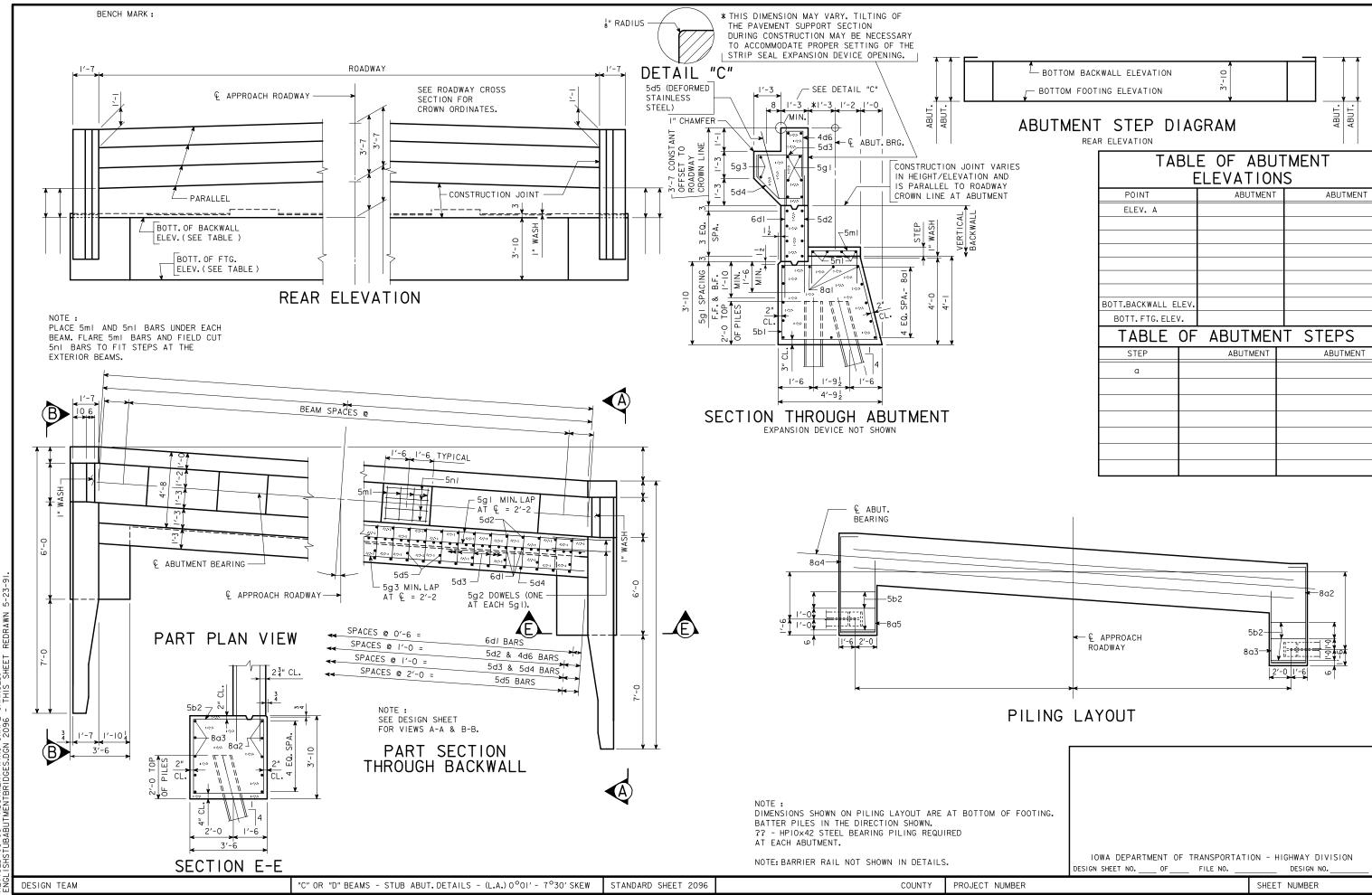


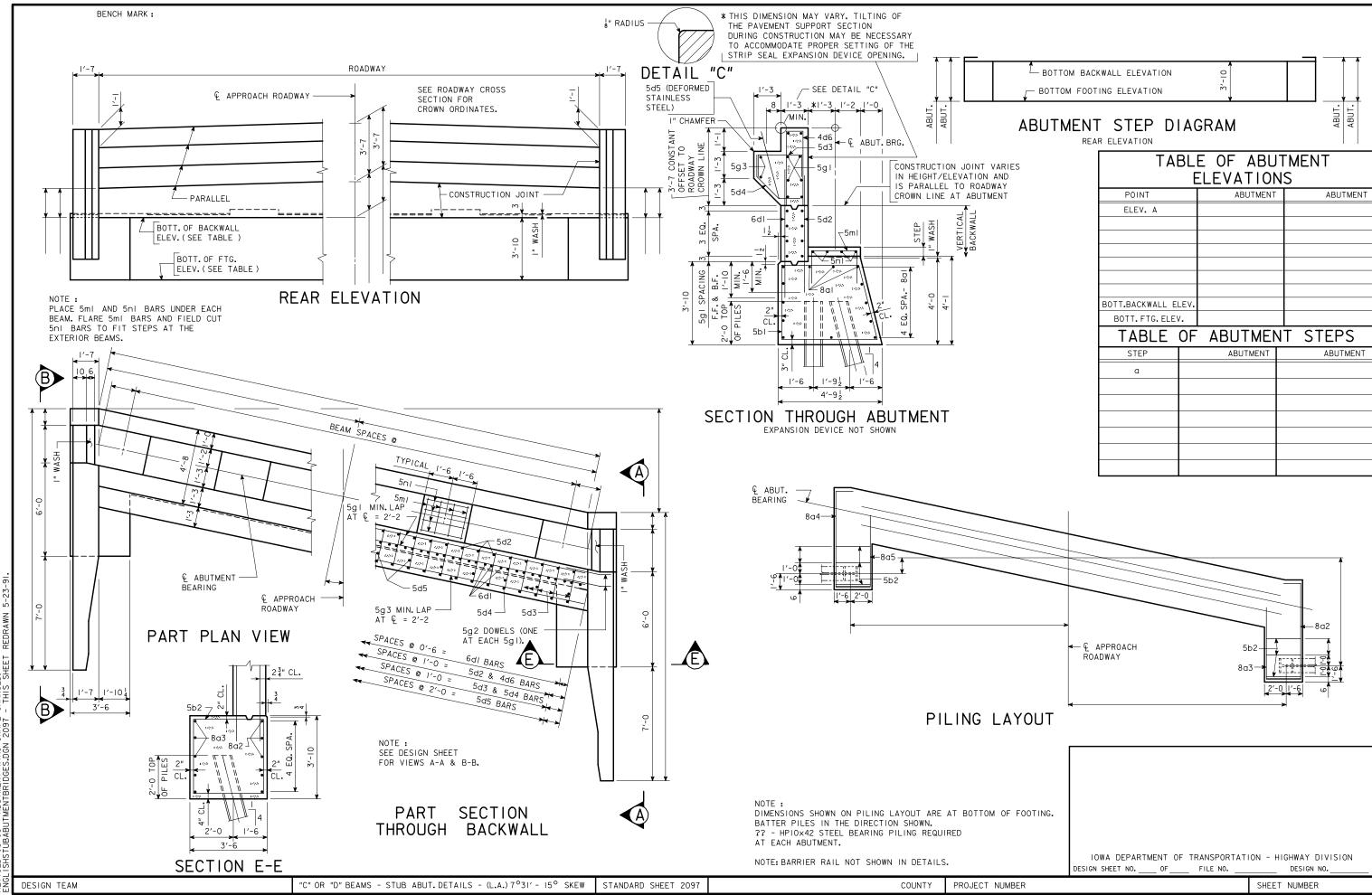


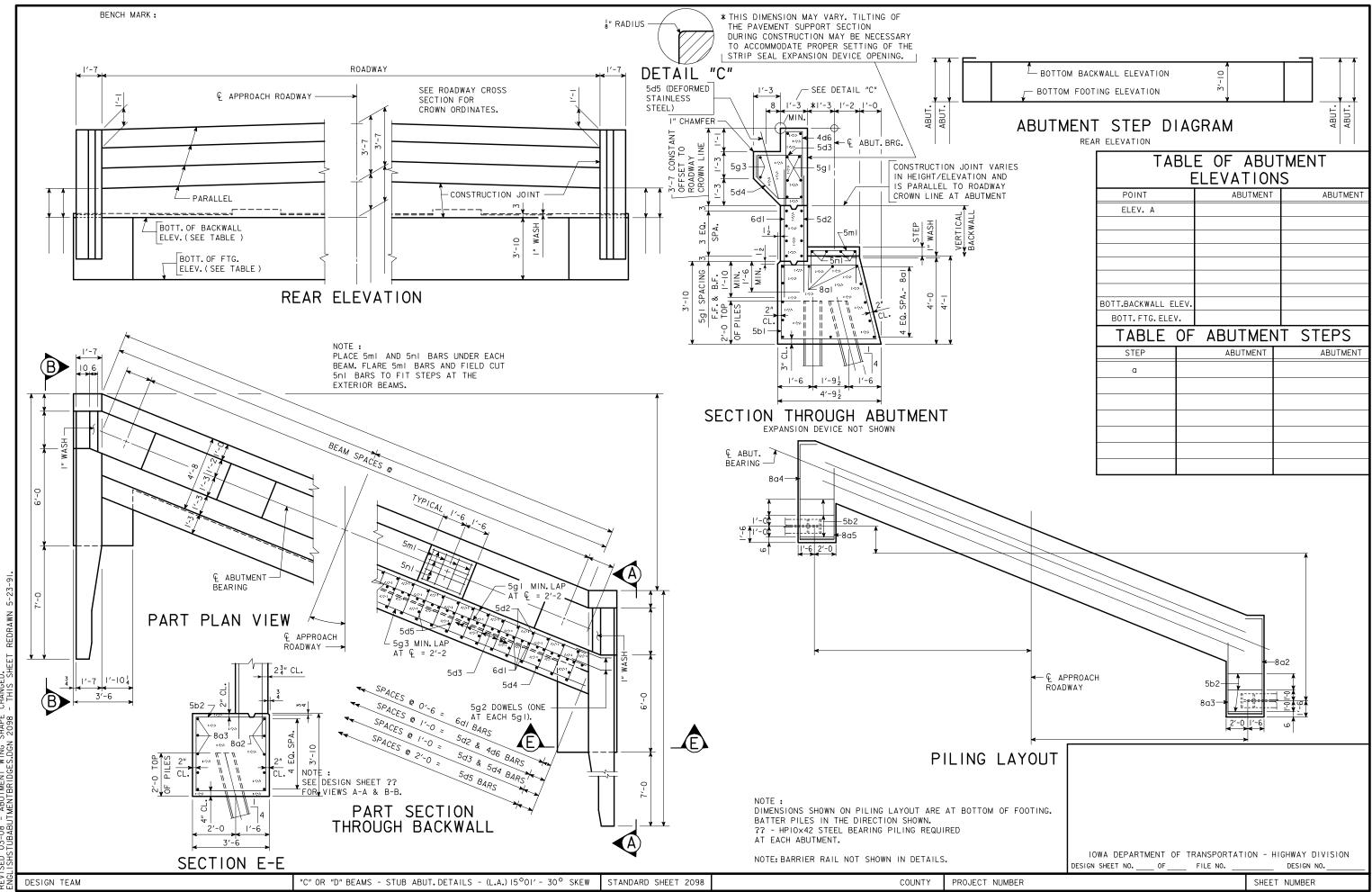


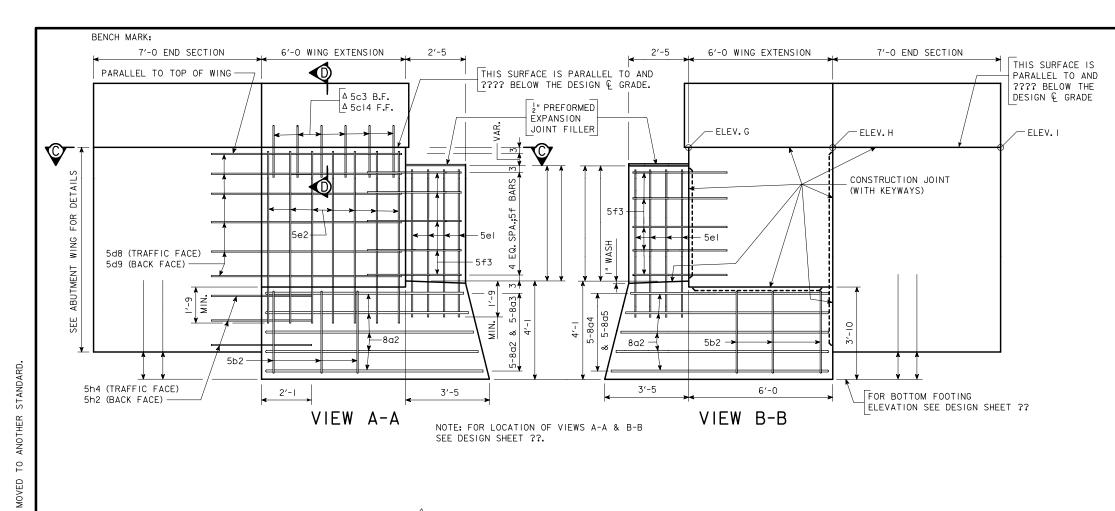


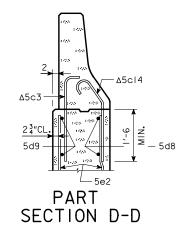












2-,		5d8 7	5f3						
	7'-0 ABUTMENT WING	5d9 5e2 6 SP. @ 0'-II = 5'-6 33 14-5e2 6'-0 WING EXTENSION	5el- 3 EQ. SP.3 8-5el 2'-5 1'-0						
	CECTION C C								

SECTION C-C NOTE: BARRIER RAIL NOT SHOWN.

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

TABLE	0F	WINGWAL	L EL	EVAT	IONS
LOCAT	ION		ELEV. G	ELEV. H	ELEV. I

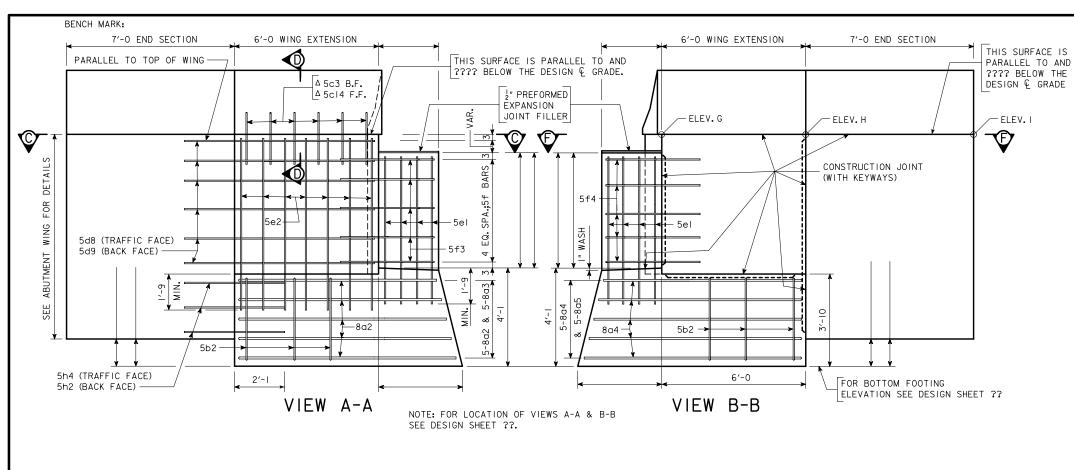
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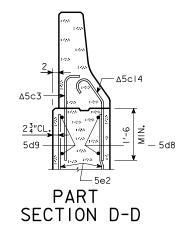
DESIGN TEAM "C" OR "D" BEAM STUB ABUT. DETAILS - 0° SKEW | STANDARD SHEET 2099 tsorens

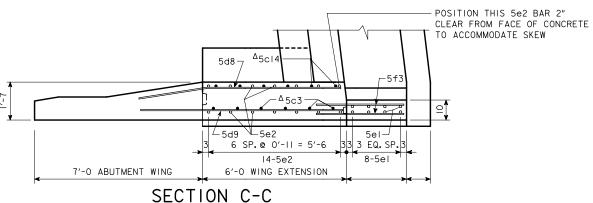
PROJECT NUMBER

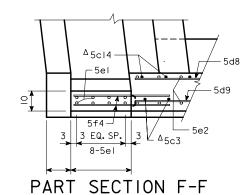
COUNTY

SHEET NUMBER









STANDARD SHEET 2100

TABLE OF WINGWALL ELEVATIONS ELEV. G ELEV. H LOCATION ELEV. I

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS.

NOTE: BARRIER RAIL NOT SHOWN.

BAR

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DESIGN TEAM

28-JAN-2010 14:18

REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

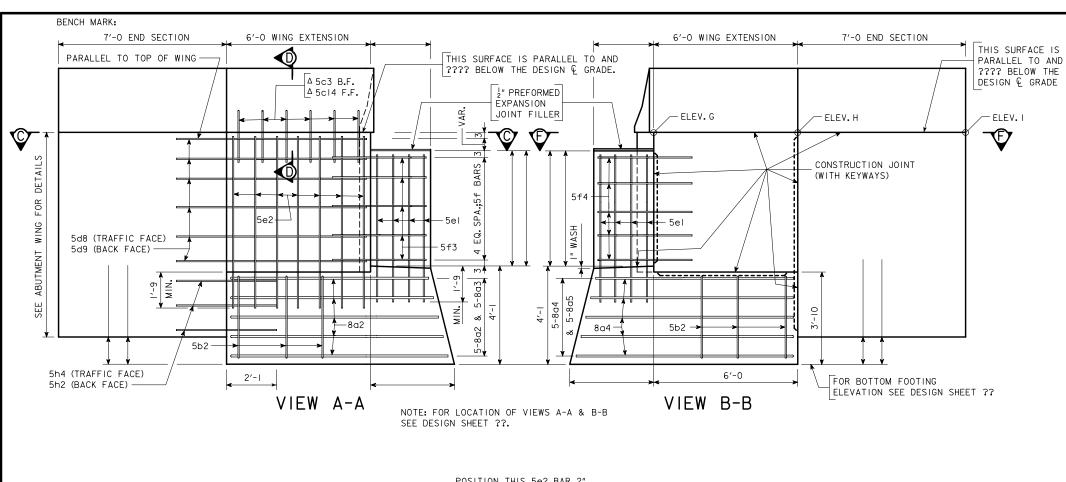
"C" OR "D" BEAM STUB ABUT. DETAILS - (R.A.)0°01' - 7°30' SKEW

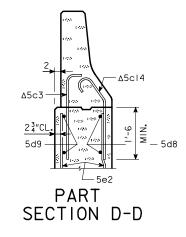
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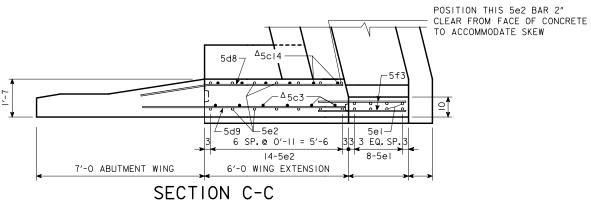
tsorens

DESIGN SHEET NO. _ _ OF _ FILE NO. DESIGN NO. PROJECT NUMBER SHEET NUMBER

COUNTY







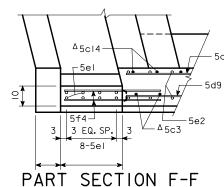


TABLE	OF	WINGWAL	L EL	EVAT	IONS
LOCATIO	N		ELEV. G	ELEV. H	ELEV. I

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

NOTE: BARRIER RAIL NOT SHOWN.

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. _ _ OF __ FILE NO. DESIGN NO.

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"C" OR "D" BEAM STUB ABUT. DETAILS - (R.A.) 7°31' - 15° SKEW

DESIGN TEAM 28-JAN-2010 14:18

BAR

∞

- ABUTMENT WING SHAPE CHANGED. QUANTITIES, NOTES, BAR LIST UTMENTBRIDGES.DGN 2101 - THIS SHEET REDRAWN 9-8-88.

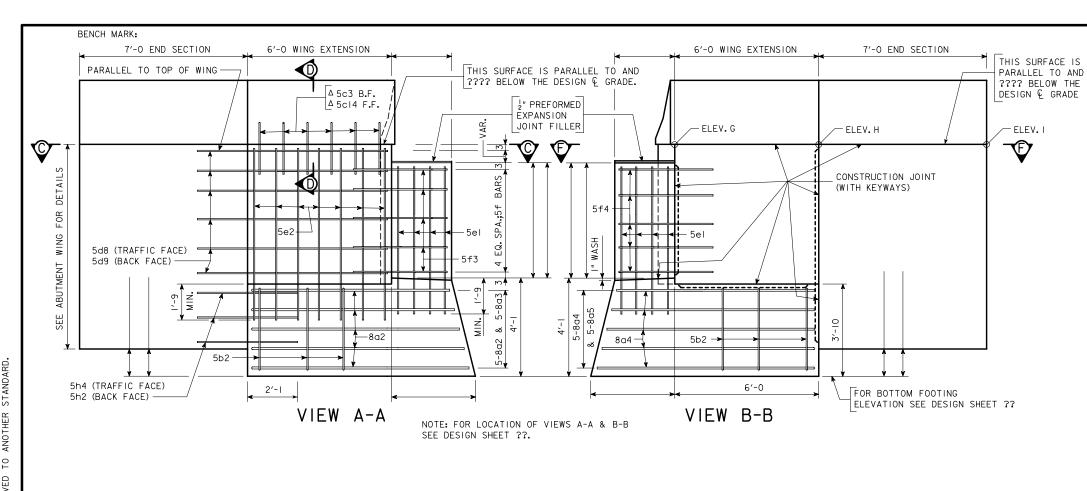
tsorens

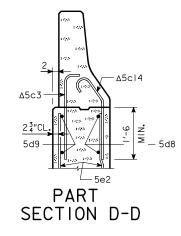
STANDARD SHEET 2101

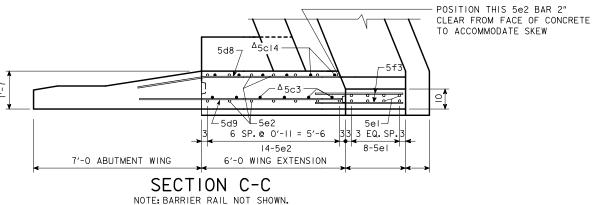
PROJECT NUMBER

COUNTY

SHEET NUMBER







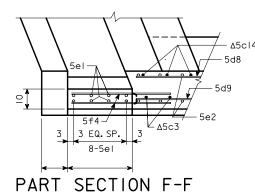


	TABLE	OF	WINGWAL	L EL	EVAT	IONS
	LOCATION	1		ELEV. G	ELEV. H	ELEV. I
I						
ŀ						
ł						
Ì						

A NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. _____ OF ____ FILE NO. _____ DESIGN NO. ____

"C" OR "D" BEAM STUB ABUT. DETAILS - (R.A.) 15°01' - 30° SKEW

STANDARD SHEET 2102

PROJECT NUMBER

COUNTY

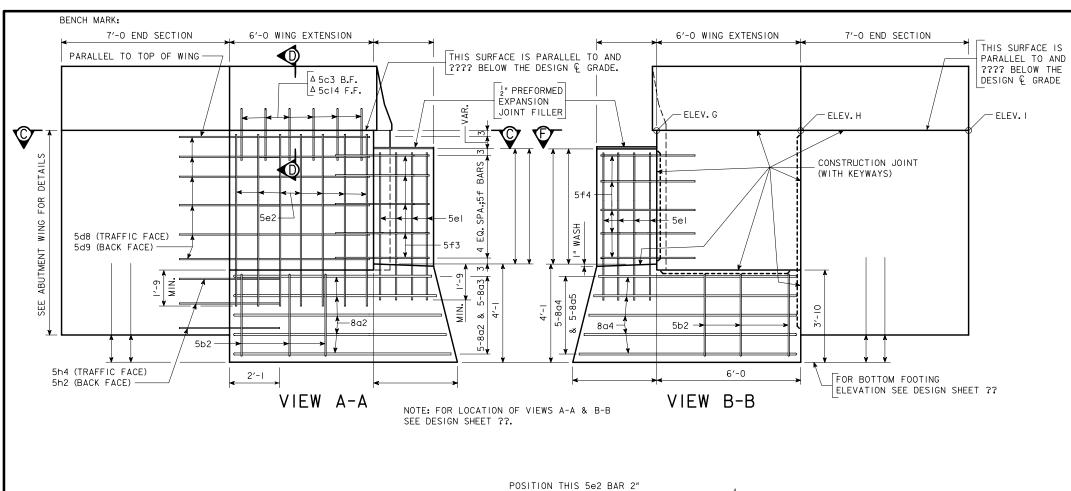
SHEET NUMBER

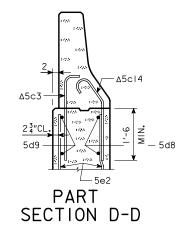
DESIGN TEAM 28-JAN-2010 14:18

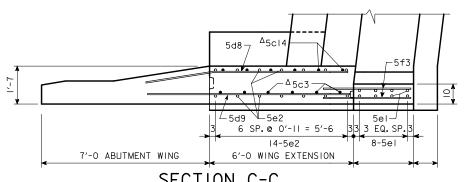
BAR

∞

03-08 - ABUTMENT WING SHAPE CHANGED, QUANTITIES, NOTES, BAR LIST TUBABUTMENTBRIDGES.DGN 2102 - THIS SHEET REDRAWN 9-8-88.







SECTION C-C NOTE: BARRIER RAIL NOT SHOWN.

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

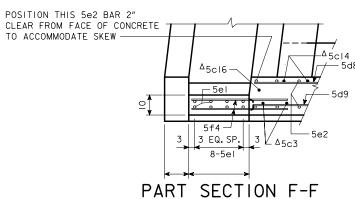


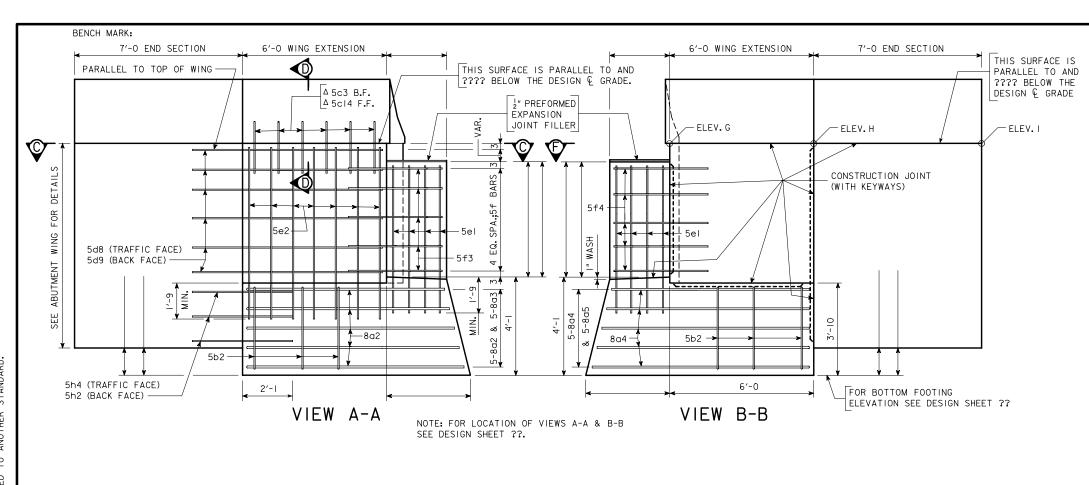
TABLE OF	WINGWAL	<u>.L EL</u>	EVATI	<u>IONS</u>
LOCATION		ELEV. G	ELEV. H	ELEV. I

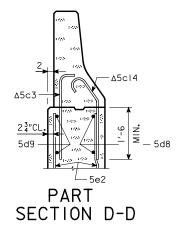
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

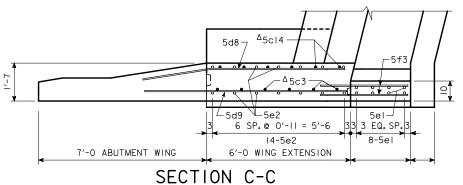
DESIGN SHEET NO. _ _ OF __ FILE NO. DESIGN NO.

COUNTY

BAR







POSITION THIS 5e2 BAR 2"
CLEAR FROM FACE OF CONCRETE
TO ACCOMMODATE SKEW

Sel

A5c1

A5c2

A5c3

A5c2

A5c3

A5c2

A5c3

A5c2

NOTE: BARRIER RAIL NOT SHOWN.

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

TABLE	OF	WINGWAL	L EL	EVAT	IONS
LOCATIO	N		ELEV. G	ELEV. H	ELEV. I

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. _____ OF ____ FILE NO. _____ DESIGN NO. ____

"C" OR "D" BEAM STUB ABUT. DETAILS - (L.A.) 7°31'-15° SKEW

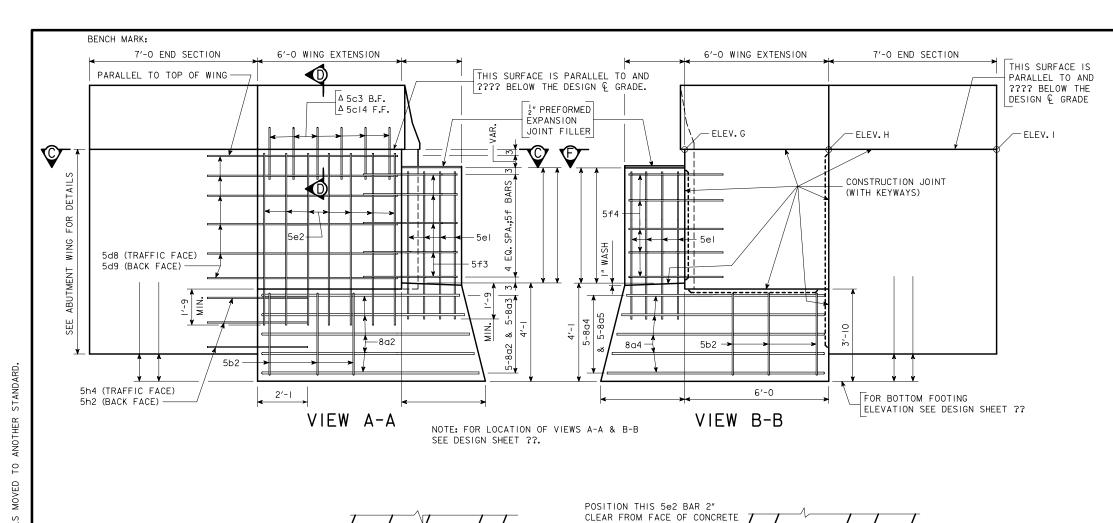
STANDARD SHEET 2104

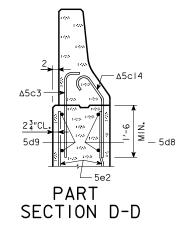
PROJECT NUMBER

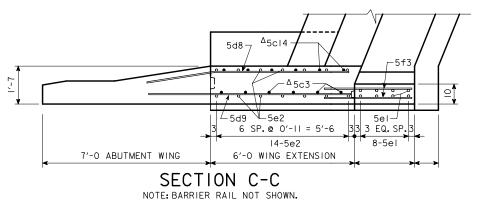
COUNTY

SHEET NUMBER

BAR







TO ACCOMMODATE SKEW -[∆]5c16 <u>√</u> 5еТ 3 3 EQ. SP. $3 \angle_{\Delta 5c3}$ 8-5el

PART SECTION F-F

TABLE OF	WINGWAL	<u>L EL</u>	EVAT	IONS
LOCATION		ELEV. G	ELEV. H	ELEV. I

Δ NOTE: SEE DESIGN SHEET ?? IN THESE PLANS FOR DETAILS OF BARRIER RAIL WING EXTENSIONS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

> IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. _ _ OF __ FILE NO. DESIGN NO.

"C" OR "D" BEAM STUB ABUT. DETAILS - (L.A.) 15°01'-30° SKEW

STANDARD SHEET 2105

PROJECT NUMBER

COUNTY

SHEET NUMBER

DESIGN TEAM 28-JAN-2010 14:18

BAR

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. THE MASKWALL IS TO BE POURED BEFORE THE SUPERSTRUCTURE SLAB IS POURED.

CONSTRUCTION JOINT KEYWAYS ARE TO BE FORMED WITH BEVELED 2x6's.

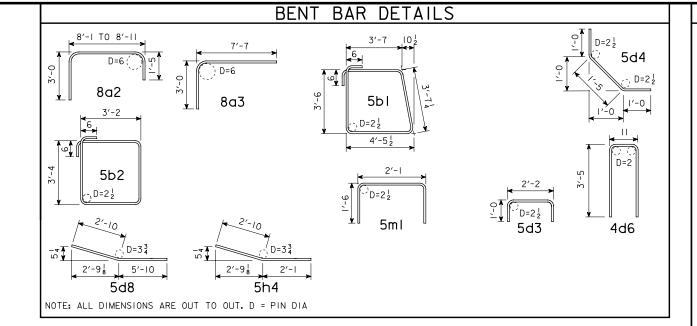
THE PORTION OF THE BACKWALL CONTAINING THE ABUTMENT ANCHORAGE OF THE EXPANSION DEVICE IS TO BE PLACED AFTER THE BRIDGE FLOOR IS PLACED.

CONCRETE SEALER IS TO BE APPLIED TO THE ABUTMENT BRIDGE SEAT IN ACCORDANCE WITH THE CURRENT IOWA D.O.T. STANDARD SPECIFICATIONS.

THE COST OF PREFORMED EXPANSION JOINT FILLER, AND COST OF FURNISHING AND PLACING CONCRETE SEALER IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)".

PAVING NOTCH DOWELS SHALL BE DEFORMED BAR GRADE 60, TYPE 316 LN IN ACCORDANCE WITH ASTM A955/A955M-OI. THE COST OF THE STAINLESS STEEL PAVING NOTCH DOWELS IS TO BE INCIDENTAL TO THE PRICE BID FOR "REINFORCING STEEL - EPOXY COATED". THE WEIGHT OF THE STAINLESS STEEL DOWELS IS TO BE INCLUDED WITH THE WEIGHT FOR EPOXY REINFORCING STEEL.

THE DESIGN BEARING FOR THE ABUTMENT PILES IS ?? TONS.
IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE
DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE
METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE
PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.



CONCRETE PLACEMENT	QUAN	TITIES
LOCATION	ABUT.	ABUT.
FOOTING AND STEPS		
BACKWALL BELOW CONSTR. JOINT		
BACKWALL ABOVE CONSTR. JOINT		
? WINGWALL		
? WINGWALL		
? WING MASKWALL		
? WING MASKWALL		
WINGS 2 @ ??? C.Y./ABUT.		
TOTAL (C.Y.)		

STANDARD SHEET 2106

ESTIMATED QUANTITIES - BOTH ABUTMENTS						
ITEM	UNIT	ABUT.	ABUT.	TOTAL		
STRUCTURAL CONCRETE (BRIDGE)	C.Y.					
REINFORCING STEEL - EPOXY COATED	LB.					
CLASS 20 EXCAVATION	C.Y.					
PILING ???	LIN.FT.					

COUNTY

PROJECT NUMBER

RE	ΙN	FORCING BAR LIST - C	NE	AΒ	UTME	NT
	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
	8al	FOOTING LONGITUDINAL				
		WING FOOTING		10	VARIES	345
	8a3	WING FOOTING		10	10'-7	283
	5b1	FOOTING HOOPS	f\		16'-2	
	5b2	WING FOOTING HOOPS	f	6	14'-0	88
ا ۱۸	6dI	BACKWALL VERTICAL B.F.				
انح	5d2	BACKWALL VERTICAL F.F.				
7	5d3	PAVING NOTCH			4'-2	
m	5d4	PAVING NOTCH	_		3′-5	
_	4d6	BACKWALL VERTICAL HOOP			7′-9	
	5d8	WING EXTENSION FF HORIZONTAL	/	12	8′-8	108
П	5d9	WING EXTENSION BF HORIZONTAL		12	8′-8	108
⊢						
⋖	5el	MASKWALL VERTICAL		16		
0	5e2	WINGWALL VERTICAL		28		
$ \circ $						
	5 f 3	MASKWALL HORIZONTAL		20	4′-3	89
)						
EPOXY COATED BARS		DAOKWALL LONGITUDINAL				
	5g I				4/ 5	100
	5g2	BACKWALL DOWELS		28	4′-5	129
	5g s	PAVING NOTCH LONGITUDINAL				
	5h2	WING TO FOOTING ANCHOR BFH		6	4′-11	31
	5h4	WING TO FOOTING ANCHOR FFH		6	4'-11	31
	דווכ	WING TO FOOTING ANCHON FFH		0	7 11	31
	5ml	BEAM STEPS TRANSVERSE			5′-1	
	0	BENIN O'LE O'NWING PENGE			J 1	
	5nl	BEAM STEPS LONGITUDINAL			2′-8	
		REINFORCING STEEL - EPOXY CO	ATED -	TOTA	L (LBS.)	
S	5d5	PAVING NOTCH DOWELS (STAINLESS STEEL)			3′-6	
اغوا						
3 ∀						
ا." ا						
S.S. BARS						
\ <u>v</u>		STAINLESS S	TEEL -	TOTA	AL (LBS.)	
						-

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. _____ OF ____ FILE NO. _____ DESIGN NO. _____

SHEET NUMBER

tsorens

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. THE MASKWALL IS TO BE POURED BEFORE THE SUPERSTRUCTURE SLAB IS POURED.

CONSTRUCTION JOINT KEYWAYS ARE TO BE FORMED WITH BEVELED 2x6's.

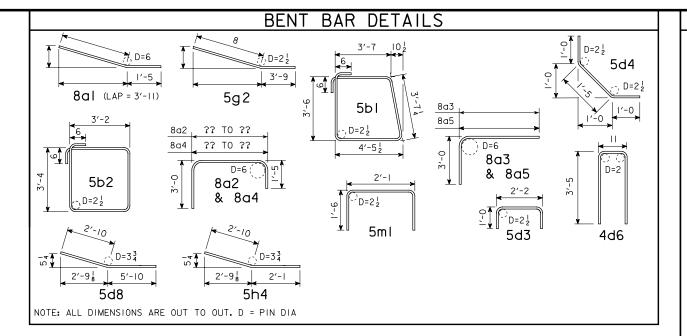
THE PORTION OF THE BACKWALL CONTAINING THE ABUTMENT ANCHORAGE OF THE EXPANSION DEVICE IS TO BE PLACED AFTER THE BRIDGE FLOOR IS PLACED.

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THE DESIGN BEARING FOR THE ABUTMENT PILES IS ?? TONS.
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DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE
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PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.



CONCRETE PLACEMENT	QUAN	TITIES
LOCATION	ABUT.	ABUT.
FOOTING AND STEPS		
BACKWALL BELOW CONSTR. JOINT		
BACKWALL ABOVE CONSTR. JOINT		
? WINGWALL		
? WINGWALL		
? WING MASKWALL		
? WING MASKWALL		
WINGS 2 @ ??? C.Y./ABUT.		
TOTAL (C.Y.)		

<u>ESTIMATED QUANTIT</u>	<u>IES</u>	- BOTH	ABUTM	ENTS
ITEM	UNIT	ABUT.	ABUT.	TOTAL
STRUCTURAL CONCRETE (BRIDGE)	C.Y.			
REINFORCING STEEL - EPOXY COATED	LB.			
CLASS 20 EXCAVATION	C.Y.			
PILING ???	LIN.FT.			

RE	INI	FORCING BAR LIST - O	NE	AB	UTME	NT
	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
	8al	FOOTING LONGITUDINAL		26		
	8a2	WING FOOTING		5	VARIES	
	8a3	WING FOOTING		5		
	8a4	WING FOOTING		5	VARIES	
	8a5	WING FOOTING		5		
	5b1	FOOTING HOOPS	<u>_</u>		16′-2	
	5b2	WING FOOTING HOOPS		6	14'-0	88
	6d1	BACKWALL VERTICAL B.F.				
BARS	5d2	BACKWALL VERTICAL F.F.				
اچ	5d2	PAVING NOTCH			4'-2	
Z I	5d4	PAVING NOTCH			3′-5	
ш	4d6	BACKWALL VERTICAL HOOP	È		7′-9	
\sim	5d8	WING EXTENSION FF HORIZONTAL		12	8′-8	108
Ш	5d9	WING EXTENSION BF HORIZONTAL		12	8′-8	108
COATED						
⋖	5eI	MASKWALL VERTICAL		16		
\circ	5e2	WINGWALL VERTICAL		28		
\circ						
		MACONINA LIBERTANITA				
\bigcirc	5f3 5f4	MASKWALL HORIZONTAL MASKWALL HORIZONTAL		10		
EPOXY	514	MASKWALL HORIZONTAL		10		
Ш	5g I	BACKWALL LONGITUDINAL				
		BACKWALL DOWELS		28	4′-5	129
)	PAVING NOTCH LONGITUDINAL			, ,	720
	Ŭ					
	5h2	WING TO FOOTING ANCHOR BFH		6	4'-11	31
	5h4	WING TO FOOTING ANCHOR FFH	/	6	4'-11	31
		DELLA CTERS TRANSVERSE				
	5ml	BEAM STEPS TRANSVERSE			5′-1	
	5nI	BEAM STEPS LONGITUDINAL			2′-8	
	3111	BEAM STEFS LUNGITUDINAL			2 -0	
		REINFORCING STEEL - EPOXY CO.	ATED -	TOTA	L (LBS.)	
S	5d5	PAVING NOTCH DOWELS (STAINLESS STEEL)			3′-6	
ايخة						
MΙ						
S.S. BARS		CTAIN FOC. C	TCC:	TOT:	1 /LDC \	
S		STAINLESS S	IEEL -	ΙΟΙΑ	r (FR2*)	

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. _____ OF ____ FILE NO. _____ DESIGN NO. ____

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. THE MASKWALL IS TO BE POURED BEFORE THE SUPERSTRUCTURE SLAB IS POURED.

CONSTRUCTION JOINT KEYWAYS ARE TO BE FORMED WITH BEVELED 2×6's.

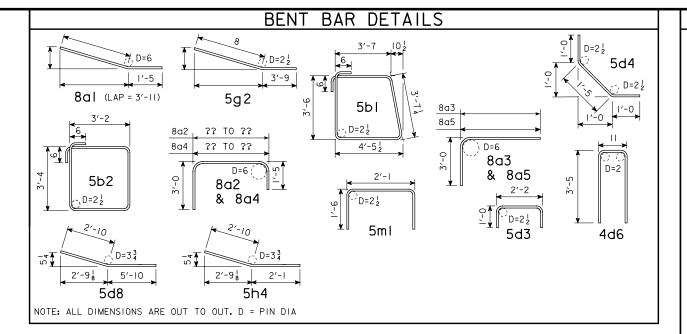
THE PORTION OF THE BACKWALL CONTAINING THE ABUTMENT ANCHORAGE OF THE EXPANSION DEVICE IS TO BE PLACED AFTER THE BRIDGE FLOOR IS PLACED.

CONCRETE SEALER IS TO BE APPLIED TO THE ABUTMENT BRIDGE SEAT IN ACCORDANCE WITH THE CURRENT IOWA D.O.T. STANDARD SPECIFICATIONS.

THE COST OF PREFORMED EXPANSION JOINT FILLER, AND COST OF FURNISHING AND PLACING CONCRETE SEALER IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)".

PAVING NOTCH DOWELS SHALL BE DEFORMED BAR GRADE 60, TYPE 316 LN IN ACCORDANCE WITH ASTM A955/A955M-OI. THE COST OF THE STAINLESS STEEL PAVING NOTCH DOWELS IS TO BE INCIDENTAL TO THE PRICE BID FOR "REINFORCING STEEL - EPOXY COATED". THE WEIGHT OF THE STAINLESS STEEL DOWELS IS TO BE INCLUDED WITH THE WEIGHT FOR EPOXY REINFORCING STEEL.

THE DESIGN BEARING FOR THE ABUTMENT PILES IS ?? TONS.
IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE
DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE
METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE
PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.



CONCRETE PLACEMENT	QUAN	TITIES
LOCATION	ABUT.	ABUT.
FOOTING AND STEPS		
BACKWALL BELOW CONSTR. JOINT		
BACKWALL ABOVE CONSTR. JOINT		
? WINGWALL		
? WINGWALL		
? WING MASKWALL		
? WING MASKWALL		
WINGS 2 @ ??? C.Y./ABUT.		
TOTAL (C.Y.)		

ESTIMATED QUANTIT	IES	- BOTH	ABUTM	ENTS
ITEM	UNIT	ABUT.	ABUT.	TOTAL
STRUCTURAL CONCRETE (BRIDGE)	C.Y.			
REINFORCING STEEL - EPOXY COATED	LB.			
CLASS 20 EXCAVATION	C.Y.			
PILING ???	LIN.FT.			

RE	INI	FORCING BAR LIST - O	NE	AB	UTME	NT
	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
	8al	FOOTING LONGITUDINAL		26		
	8a2	WING FOOTING		5	VARIES	
	8a3	WING FOOTING		5		
	8a4	WING FOOTING		5	VARIES	
	8a5	WING FOOTING		5		
	5b1	FOOTING HOOPS			16′-2	
	5b2	WING FOOTING HOOPS		6	14'-0	88
	6 11	210////				
S	6d1	BACKWALL VERTICAL B.F.				
\propto	5d2	BACKWALL VERTICAL F.F.			4/ 0	
٧	5d3	PAVING NOTCH			4′-2	
BARS	5d4	PAVING NOTCH	$\stackrel{\sim}{=}$		3′-5 7′-9	
	4d6	BACKWALL VERTICAL HOOP WING EXTENSION FF HORIZONTAL		13	8'-8	100
	5d8 5d9	WING EXTENSION OF HORIZONTAL	$\stackrel{\sim}{=}$	12	8′-8	108
1	303	WING EXTENSION OF HORIZONTAL		12	0 -0	100
`∀	5el	MASKWALL VERTICAL		16		
\hat{C}	5e2			28		
$\ddot{\circ}$		WHO WHEE TENT TO THE				
\succ						
\times	5 f 3	MASKWALL HORIZONTAL		10		
Ó	5f4	MASKWALL HORIZONTAL		10		
EPOXY COATED						
ш	5g I	BACKWALL LONGITUDINAL	_			
)	BACKWALL DOWELS		28	4′-5	129
	5g3	PAVING NOTCH LONGITUDINAL				
	5h2	WING TO FOOTING ANCHOR BFH		6	4′-11	31
	5h4	WING TO FOOTING ANCHOR FFH	_	6	4′-11	31
	5ml	BEAM STEPS TRANSVERSE			5′-1	
	JIIII	BLAW STELS THANSVERSE			3 -1	
	5nI	BEAM STEPS LONGITUDINAL			2′-8	
	3111	BEAM STEES ECHOTTOBINAL				
		REINFORCING STEEL - EPOXY CO.	ATED -	TOTA	L (LBS.)	
S	5d5				3′-6	
انج						
34						
٦.						
S.S. BARS						
S		STAINLESS S	TEEL -	TOTA	L (LBS.)	

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION
DESIGN SHEET NO. _____ OF ____ FILE NO. _____ DESIGN NO. ____

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN. THE MASKWALL IS TO BE POURED BEFORE THE SUPERSTRUCTURE SLAB IS POURED.

CONSTRUCTION JOINT KEYWAYS ARE TO BE FORMED WITH BEVELED

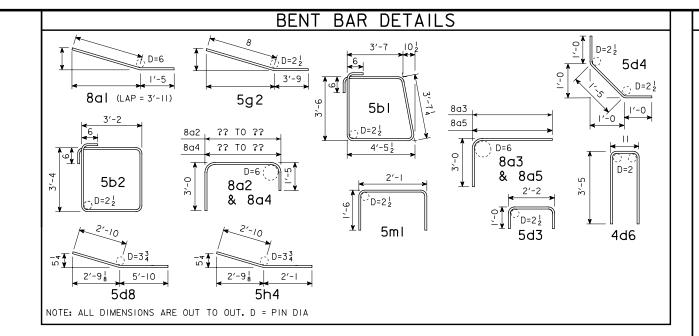
THE PORTION OF THE BACKWALL CONTAINING THE ABUTMENT ANCHORAGE OF THE EXPANSION DEVICE IS TO BE PLACED AFTER THE

CONCRETE SEALER IS TO BE APPLIED TO THE ABUTMENT BRIDGE SEAT IN ACCORDANCE WITH THE CURRENT IOWA D.O.T. STANDARD SPECIFICATIONS.

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THE DESIGN BEARING FOR THE ABUTMENT PILES IS ?? TONS. IF NECESSARY TO PREVENT DAMAGE TO THE END OF THE BRIDGE DECK OR BACKWALL FROM CONSTRUCTION EQUIPMENT, AN APPROPRIATE METHOD OF PROTECTION APPROVED BY THE ENGINEER SHALL BE PROVIDED BY THE BRIDGE CONTRACTOR AT NO EXTRA COST TO THE STATE.



CONCRETE PLACEMENT	[QUAN]	TITIES
LOCATION	ABUT.	ABUT.
FOOTING AND STEPS		
BACKWALL BELOW CONSTR.JOINT		
BACKWALL ABOVE CONSTR. JOINT		
? WINGWALL		
? WINGWALL		
? WING MASKWALL		
? WING MASKWALL		
WINGS 2 @ ??? C.Y./ABUT.		
TOTAL (C.Y.)		
? WINGWALL ? WING MASKWALL ? WING MASKWALL WINGS 2 @ ??? C.Y. / ABUT.		

ESTIMATED QUANTITIES - BOTH ABUTMENTS				
ITEM	UNIT	ABUT.	ABUT.	TOTAL
STRUCTURAL CONCRETE (BRIDGE)	C.Y.			
REINFORCING STEEL - EPOXY COATED	LB.			
CLASS 20 EXCAVATION	C.Y.			
PILING ???	LIN.FT.			

RE	INI	FORCING BAR LIST - O	NE	AB	UTME	NT
	BAR	LOCATION	SHAPE	NO.	LENGTH	WEIGHT
	8al	FOOTING LONGITUDINAL		26		
	8a2	WING FOOTING		5	VARIES	
	8a3	WING FOOTING		5		
	8a4	WING FOOTING		5	VARIES	
	8a5	WING FOOTING		5		
	5b1	FOOTING HOOPS	<u>_</u>		16′-2	
	5b2	WING FOOTING HOOPS		6	14'-0	88
	6d1	BACKWALL VERTICAL B.F.				
BARS	5d2	BACKWALL VERTICAL F.F.				
اچ	5d2	PAVING NOTCH			4'-2	
Z I	5d4	PAVING NOTCH			3′-5	
ш	4d6	BACKWALL VERTICAL HOOP	È		7′-9	
\sim	5d8	WING EXTENSION FF HORIZONTAL		12	8′-8	108
Ш	5d9	WING EXTENSION BF HORIZONTAL		12	8′-8	108
COATED						
⋖	5eI	MASKWALL VERTICAL		16		
\circ	5e2	WINGWALL VERTICAL		28		
\circ						
		MARKET HORIZONTH				
\bigcirc	5f3 5f4	MASKWALL HORIZONTAL MASKWALL HORIZONTAL		10		
EPOXY	514	MASKWALL HORIZONTAL		10		
Ш	5g I	BACKWALL LONGITUDINAL				
		BACKWALL DOWELS		28	4′-5	129
)	PAVING NOTCH LONGITUDINAL			, ,	720
	Ŭ					
	5h2	WING TO FOOTING ANCHOR BFH		6	4'-11	31
	5h4	WING TO FOOTING ANCHOR FFH	/	6	4'-11	31
		DELLA CTERS TRANSVERSE				
	5ml	BEAM STEPS TRANSVERSE			5′-1	
	5nI	BEAM STEPS LONGITUDINAL			2′-8	
	3111	BEAM STEFS LUNGITUDINAL			2 -0	
		REINFORCING STEEL - EPOXY CO.	ATED -	TOTA	L (LBS.)	
S	5d5	PAVING NOTCH DOWELS (STAINLESS STEEL)			3′-6	
ايخة						
MΙ						
S.S. BARS		CTAIN FOC. C	TCC:	TOT:	1 /LDC \	
S		STAINLESS S	IEEL -	ΙΟΙΑ	r (FR2*)	

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. ___ ___ OF ___ FILE NO. DESIGN NO.

TRANSVERSE SLAB CONSTRUCTION JOINT

SUPERSTRUCTURE NOTES:

THE FLOOR SLAB AS SHOWN INCLUDES 2" INTEGRAL WEARING SURFACE.

FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE GIRDERS.

CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.

TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND 21" CLEAR BELOW TOP OF SLAB, BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND I" CLEAR ABOVE BOTTOM OF SLAB. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-0 CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4'-0 APART.

ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH TENSILE STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE $_{16}^{15}$ " φ AND ALL BOLTS ARE TO BE $_{8}^{7}$ " φ .

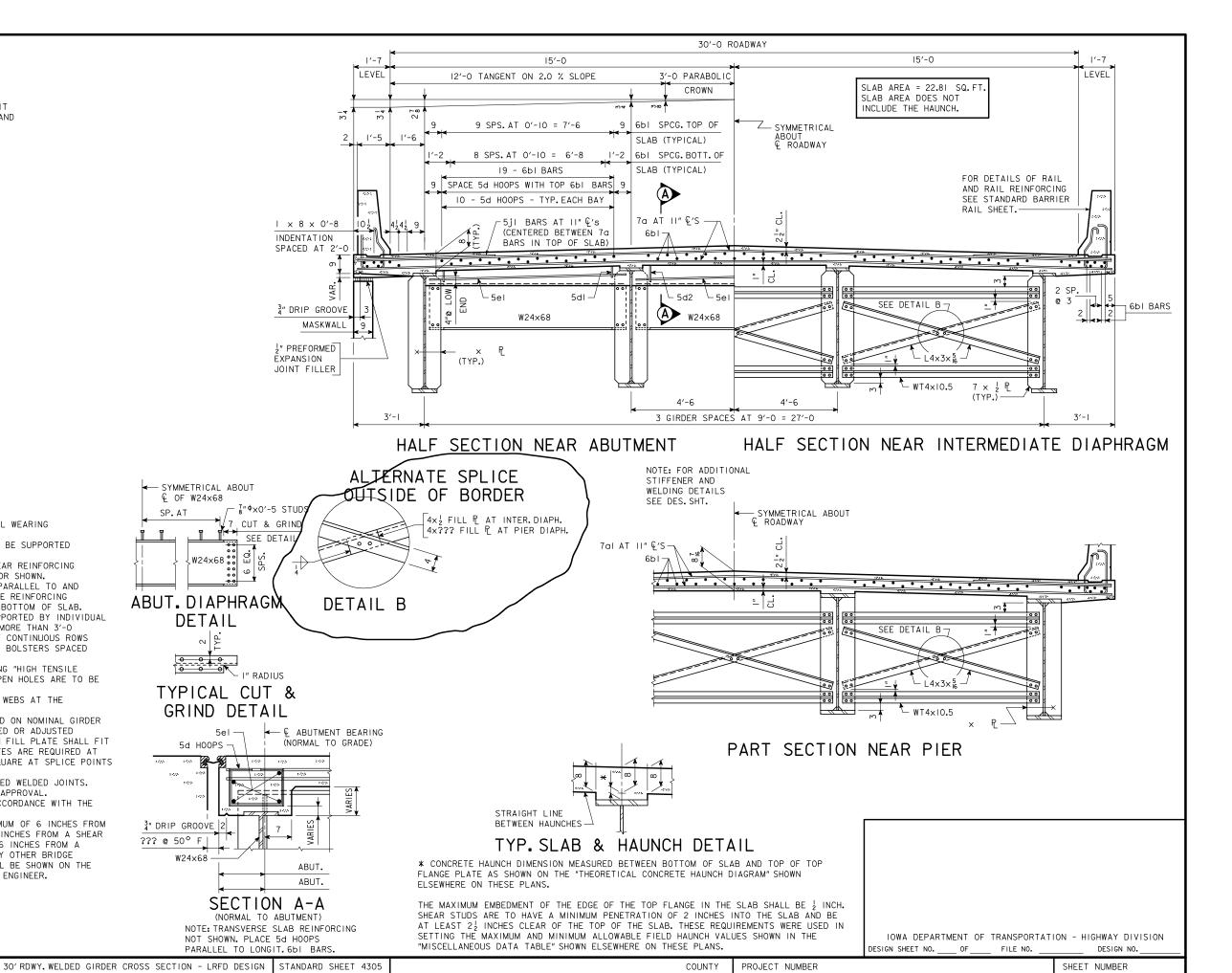
BOTTOM FLANGES ARE TO BE PERPENDICULAR TO WEBS AT THE REACTION POINTS.

FILL P THICKNESSES SHOWN ON PLANS ARE BASED ON NOMINAL GIRDER DIMENSIONS. THESE THICKNESSES ARE TO BE VERIFIED OR ADJUSTED DURING FABRICATION TO SECURE A CLOSE FIT. EACH FILL PLATE SHALL FIT TO THE NEAREST 16" IN THICKNESS AND SINGLE PLATES ARE REQUIRED AT EACH FILL LOCATION. GIRDERS ARE TO BE TRULY SQUARE AT SPLICE POINTS WITH FLANGES PERPENDICULAR TO WEBS.

THE DESIGN DRAWINGS INDICATE AWS PREQUALIFIED WELDED JOINTS. ALTERNATE JOINT DETAILS MAY BE SUBMITTED FOR APPROVAL.

MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, WILL BE REQUIRED.

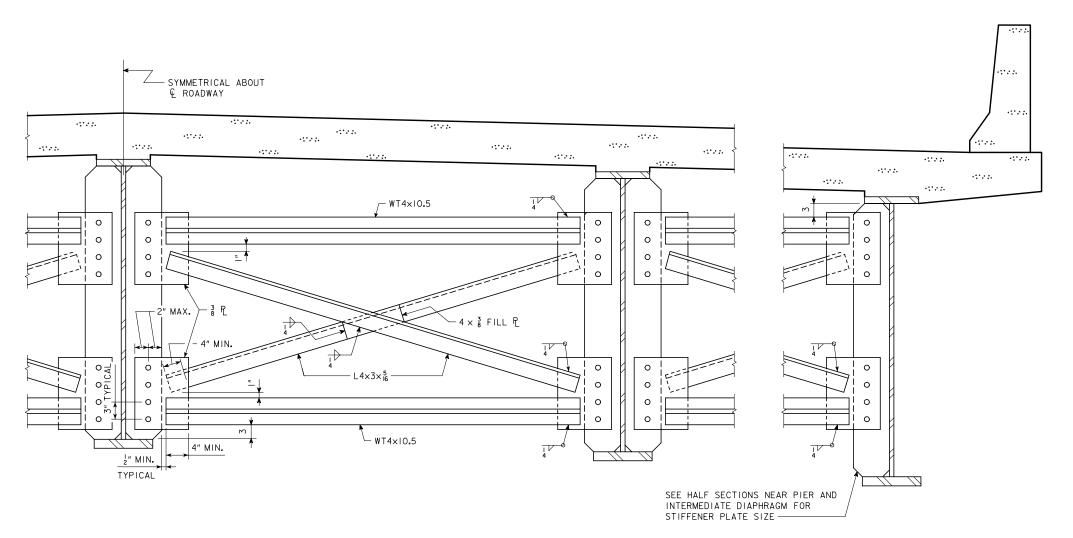
SHOP WELDED FLANGE SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER, 6 INCHES FROM A WEB SPLICE, AND 4 INCHES FROM A SHEAR CONNECTOR. WEB SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER. SPLICES SHALL NOT INTERFERE WITH ANY OTHER BRIDGE COMPONENTS. ALL SHOP WELDED BUTT SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS AND SUBJECT TO APPROVAL BY THE ENGINEER.



??? @ 50°

W24×68

DESIGN TEAM



ALTERNATE INTERMEDIATE DIAPHRAGM PART SECTION THRU SLAB

(SHOWING ONE DIAPHRAGM BETWEEN GIRDERS)

THIS CANNOT BE WELDED FROM ONE SIDE, CROSS FRAME MUST BE TURNED OVER TO ADD SECOND ANGLE.

ALTERNATE INTERMEDIATE DIAPHRAGM NOTES:

ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH TENSILE STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE 15"Φ AND ALL BOLTS ARE TO BE 7"Φ.

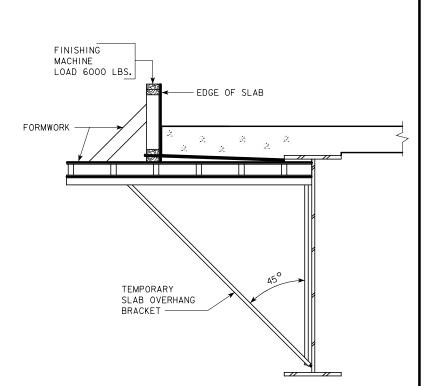
THE DESIGN DRAWINGS INDICATE AWS PREQUALIFIED WELDED JOINTS.

ALTERNATE JOINT DETAILS MAY BE SUBMITTED FOR APPROVAL.

MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE

STANDARD SPECIFICATIONS, WILL BE REQUIRED.

STRUCTURAL STEEL QUANTITIES ARE BASED ON THE INTERMEDIATE DIAPHRAGM SHOWN ON TYPICAL CROSS SECTION ELSEWHERE IN THESE PLANS. NO ADJUSTMENT TO QUANTITIES WILL BE MADE IF THE CONTRACTOR USES THIS ALTERNATE INTERMEDIATE DIAPHRAGM DETAIL.



TEMPORARY SLAB OVERHANG BRACKET DETAIL

OVERHANG BRACKET NOTES:

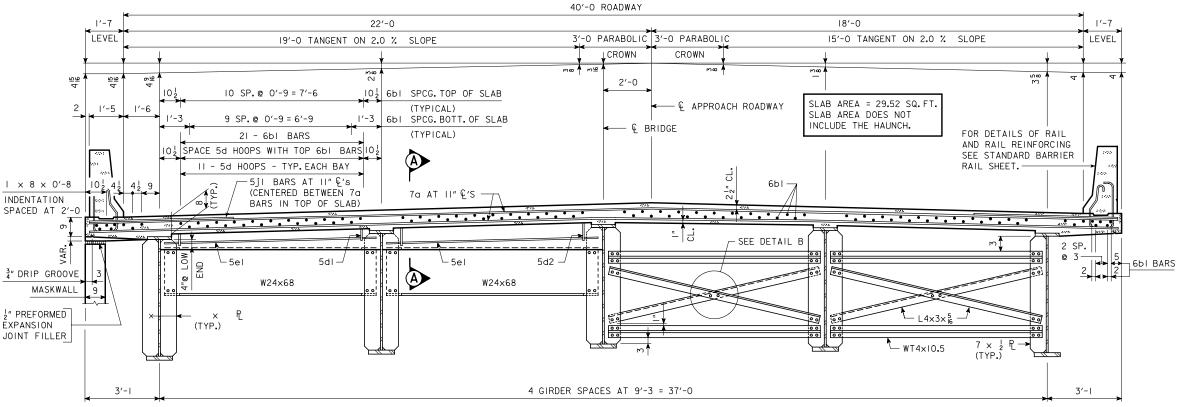
A MAXIMUM FINISHING MACHINE LOAD AND THE ANGLE OF THE DIAGONAL MEMBER OF THE OVERHANG BRACKET SHOWN WERE ASSUMED BY THE DESIGNER. THESE ASSUMPTIONS, IN ADDITION TO OTHER CONSTRUCTION LOADINGS, WERE USED TO CHECK THE STRENGTH OF THE EXTERIOR GIRDER DURING CRITICAL STAGES OF CONSTRUCTION, IF THE FINISHING MACHINE LOAD OR ANGLE OF THE DIAGONAL MEMBER OF THE OVERHANG BRACKET DEVIATE SIGNIFICANTLY FROM VALUES SHOWN, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THIS INFORMATION ON PROPOSED CONSTRUCTION EQUIPMENT TO BE USED.

IF THE VERTICAL HEIGHT OF THE OVERHANG BRACKET IS ADJUSTABLE, THE BASE OF THE BRACKET IS TO BE LOCATED AS CLOSE AS POSSIBLE TO THE BOTTOM FLANGE OF THE GIRDER.

ALT. DIAPH. & TEMP. OVERHANG BRACKET

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION 0F DESIGN NO.

DESIGN SHEET NO. _ FILE NO. TEMP. OVERHANG BRACKET & ALT. INTERM. DIAPH. FOR WELDED GIRDER BRIDGES - LRFD DESIGN STANDARD SHEET 4305A COUNTY PROJECT NUMBER SHEET NUMBER TRANSVERSE SLAB CONSTRUCTION JOINT



SUPERSTRUCTURE NOTES:

THE FLOOR SLAB AS SHOWN INCLUDES $\frac{1}{2}{}^{\prime\prime}$ INTEGRAL WEARING SURFACE.

FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE GIRDERS.

CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.

TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND $2\frac{1}{2}$ CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND I" CLEAR ABOVE BOTTOM OF SLAB. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-O CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4'-O APART.

TRANSVERSE SLAB REINFORCING MAY BE SPLICED WITH ONE LAP LOCATED AS FOLLOWS:

TOP BAR - LAP MIDWAY BETWEEN BEAMS (MIN.LAP = 3'-3). BOTTOM BARS - LAP OVER BEAMS (MIN.LAP = 3'-3). PAYMENT FOR REINFORCING BARS SHALL BE BASED ON NO SPLICES, AND NO ALLOWANCE SHALL BE MADE FOR THE ADDITIONAL LENGTH OF BAR REQUIRED FOR THE USE OF SPLICES.

ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE \$\frac{1}{2}\text{"}\Phi\$ AND ALL BOLTS ARE TO BE \$\frac{1}{2}\text{"}\Phi\$.

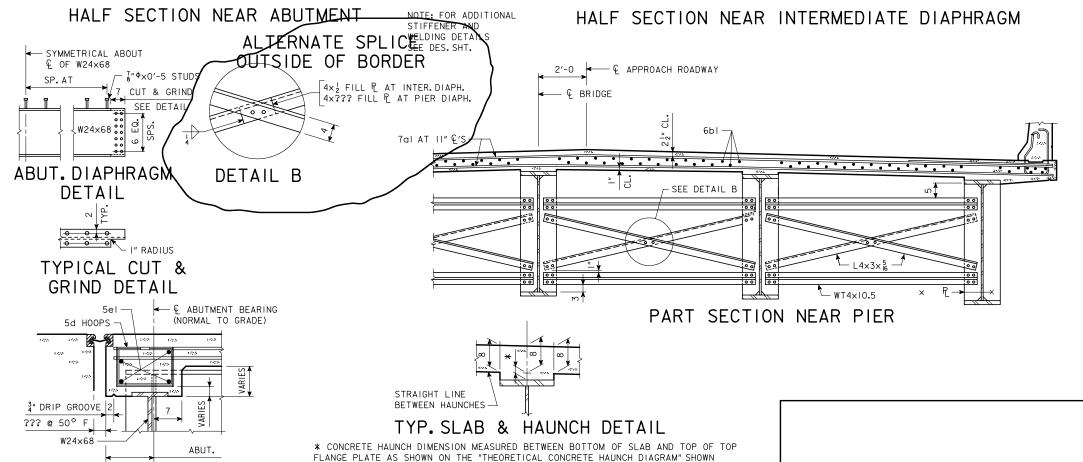
TO BE 15" AND ALL BOLTS ARE TO BE 7" A. BOTTOM FLANGES ARE TO BE PERPENDICULAR TO WEBS AT THE

FILL THICKNESSES SHOWN ON PLANS ARE BASED ON NOMINAL GIRDER DIMENSIONS. THESE THICKNESSES ARE TO BE VERIFIED OR ADJUSTED DURING FABRICATION TO SECURE A CLOSE FIT. EACH FILL PLATE SHALL FIT TO THE NEAREST "6" IN THICKNESS AND SINGLE PLATES ARE REQUIRED AT EACH FILL LOCATION. GIRDERS ARE TO BE TRULY SQUARE AT SPLICE POINTS WITH FLANGES PERPENDICULAR TO WEBS.

THE DESIGN DRAWINGS INDICATE AWS PREQUALIFIED WELDED JOINTS. ALTERNATE JOINT DETAILS MAY BE SUBMITTED FOR APPROVAL.

MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, WILL BE REQUIRED.

SHOP WELDED FLANGE SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER, 6 INCHES FROM A WEB SPLICE, AND 4 INCHES FROM A SHEAR CONNECTOR. WEB SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER. SPLICES SHALL NOT INTERFERE WITH ANY OTHER BRIDGE COMPONENTS. ALL SHOP WELDED BUTT SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS AND SUBJECT TO APPROVAL BY THE ENGINEER.



THE MAXIMUM EMBEDMENT OF THE EDGE OF THE TOP FLANGE IN THE SLAB SHALL BE 1 INCH.

AT LEAST 21 INCHES CLEAR OF THE TOP OF THE SLAB. THESE REQUIREMENTS WERE USED IN

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

DESIGN NO.

SHEET NUMBER

FILE NO.

DESIGN SHEET NO.

PROJECT NUMBER

SHEAR STUDS ARE TO HAVE A MINIMUM PENETRATION OF 2 INCHES INTO THE SLAB AND BE

SETTING THE MAXIMUM AND MINIMUM ALLOWABLE FIELD HAUNCH VALUES SHOWN IN THE

"MISCELLANEOUS DATA TABLE" SHOWN ELSEWHERE ON THESE PLANS.

ABUT.

SECTION A-A

(NORMAL TO ABUTMENT)

NOTE: TRANSVERSE SLAB REINFORCING

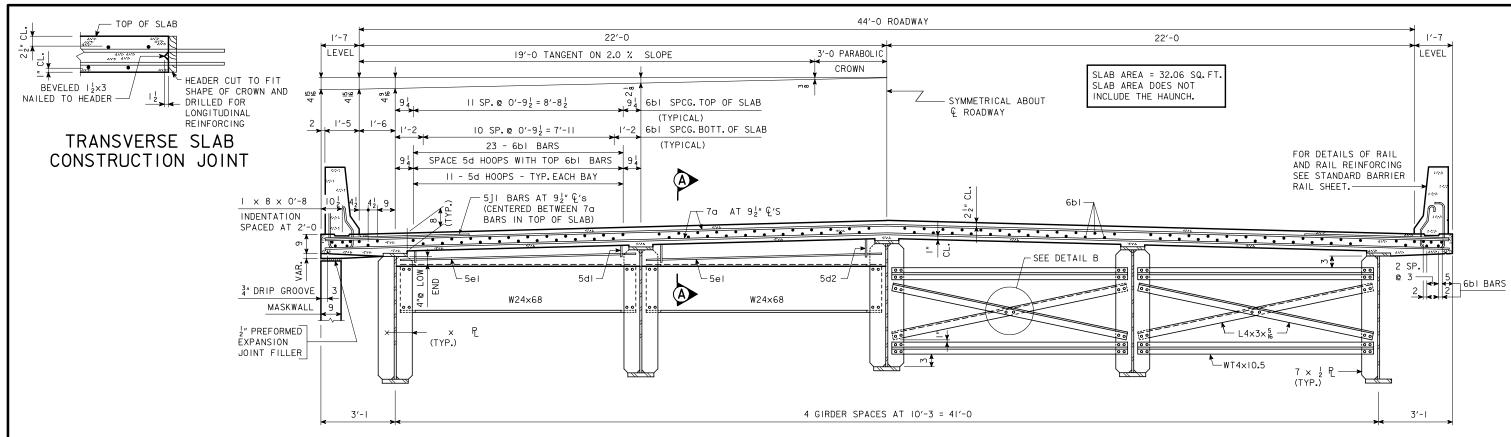
NOT SHOWN. PLACE 5d HOOPS

40' RDWY. WELDED GIRDER CROSS SECTION - LRFD DESIGN | STANDARD SHEET 4308

PARALLEL TO LONGIT. 6b1 BARS

ELSEWHERE ON THESE PLANS.

REACTION POINTS.



SUPERSTRUCTURE NOTES:

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ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE 15" AND ALL BOLTS ARE TO BE 7" A.

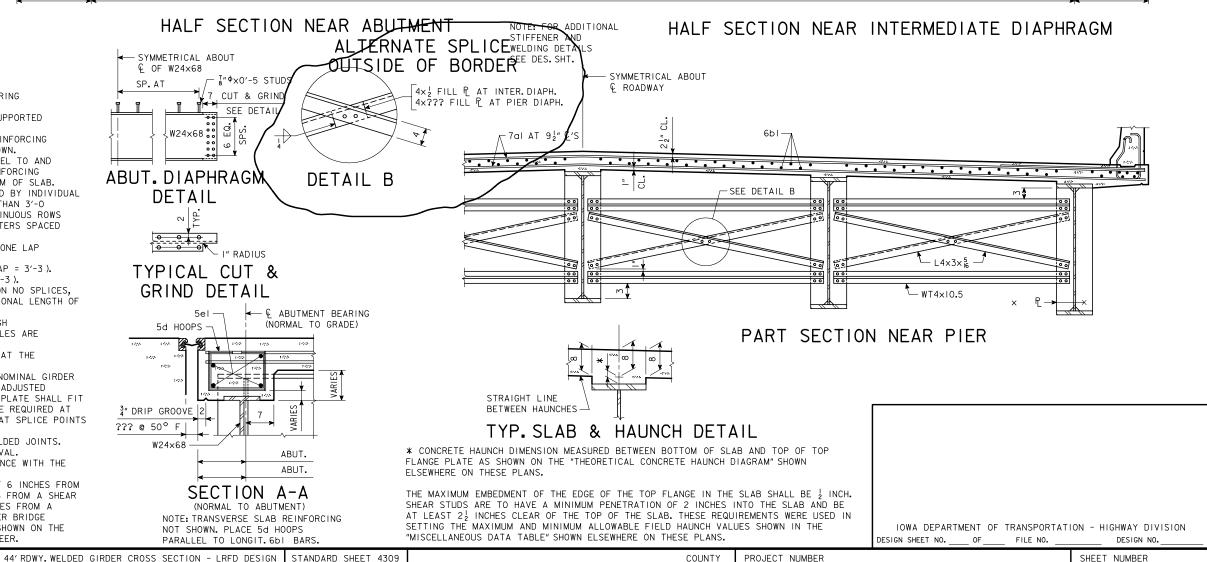
BOTTOM FLANGES ARE TO BE PERPENDICULAR TO WEBS AT THE REACTION POINTS.

FILL P THICKNESSES SHOWN ON PLANS ARE BASED ON NOMINAL GIRDER DIMENSIONS. THESE THICKNESSES ARE TO BE VERIFIED OR ADJUSTED DURING FABRICATION TO SECURE A CLOSE FIT. EACH FILL PLATE SHALL FIT TO THE NEAREST 16" IN THICKNESS AND SINGLE PLATES ARE REQUIRED AT EACH FILL LOCATION. GIRDERS ARE TO BE TRULY SQUARE AT SPLICE POINTS WITH FLANGES PERPENDICULAR TO WEBS.

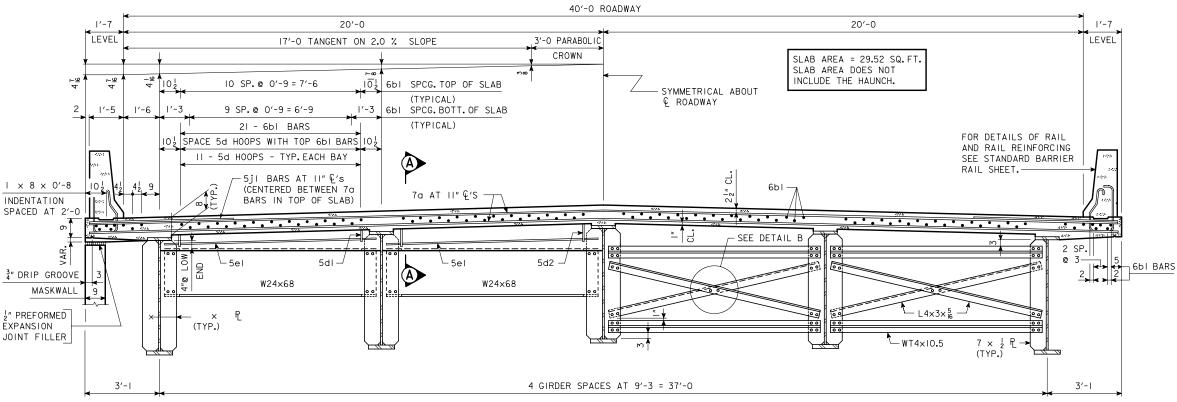
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MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, WILL BE REQUIRED.

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TRANSVERSE SLAB CONSTRUCTION JOINT



SUPERSTRUCTURE NOTES:

THE FLOOR SLAB AS SHOWN INCLUDES $\frac{1}{2}{}^{\prime\prime}$ INTEGRAL WEARING SURFACE.

FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE GIRDERS.

CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN.

TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND $2\frac{1}{2}$ CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND I" CLEAR ABOVE BOTTOM OF SLAB. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 3^\prime -O CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4^\prime -O APART.

TRANSVERSE SLAB REINFORCING MAY BE SPLICED WITH ONE LAP LOCATED AS FOLLOWS:

TOP BAR - LAP MIDWAY BETWEEN BEAMS (MIN.LAP = 3'-3).
BOTTOM BARS - LAP OVER BEAMS (MIN.LAP = 3'-3).
PAYMENT FOR REINFORCING BARS SHALL BE BASED ON NO SPLICES,
AND NO ALLOWANCE SHALL BE MADE FOR THE ADDITIONAL LENGTH OF
BAR REQUIRED FOR THE USE OF SPLICES.

ALL FIELD CONNECTIONS ARE TO BE BOLTED USING "HIGH STRENGTH BOLTS". UNLESS OTHERWISE NOTED, ALL OPEN HOLES ARE TO BE \$\frac{1}{6}" \Phi \text{ AND ALL BOLTS ARE TO BE \$\frac{1}{6}" \Phi \text{.}}

BOTTOM FLANGES ARE TO BE PERPENDICULAR TO WEBS AT THE REACTION POINTS.

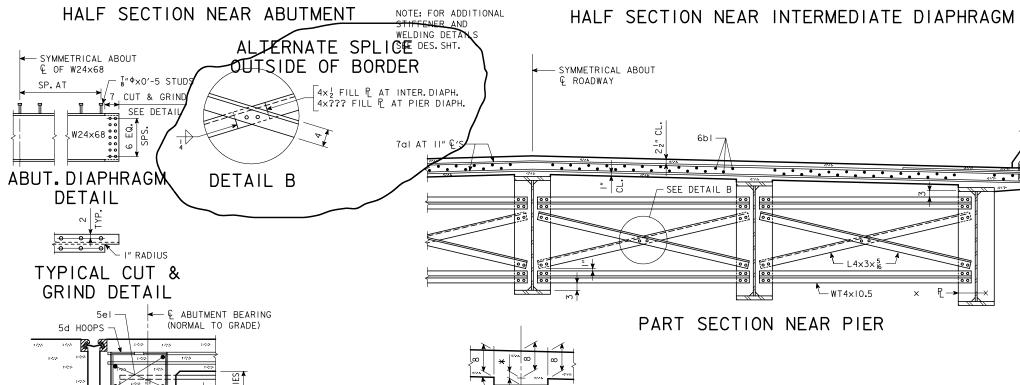
FILL THICKNESSES SHOWN ON PLANS ARE BASED ON NOMINAL GIRDER DIMENSIONS. THESE THICKNESSES ARE TO BE VERIFIED OR ADJUSTED DURING FABRICATION TO SECURE A CLOSE FIT. EACH FILL PLATE SHALL FIT TO THE NEAREST 16" IN THICKNESS AND SINGLE PLATES ARE REQUIRED AT EACH FILL LOCATION. GIRDERS ARE TO BE TRULY SQUARE AT SPLICE POINTS WITH FLANGES PERPENDICULAR TO WEBS.

THE DESIGN DRAWINGS INDICATE AWS PREQUALIFIED WELDED JOINTS. ALTERNATE JOINT DETAILS MAY BE SUBMITTED FOR APPROVAL.

MAGNETIC PARTICLE INSPECTION OF WELDS, IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS, WILL BE REQUIRED.

SHOP WELDED FLANGE SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER, 6 INCHES FROM A WEB SPLICE, AND 4 INCHES FROM A SHEAR CONNECTOR. WEB SPLICES SHALL BE A MINIMUM OF 6 INCHES FROM A STIFFENER. SPLICES SHALL NOT INTERFERE WITH ANY OTHER BRIDGE COMPONENTS. ALL SHOP WELDED BUTT SPLICES SHALL BE SHOWN ON THE SHOP DRAWINGS AND SUBJECT TO APPROVAL BY THE ENGINEER.

tsorens



TYP. SLAB & HAUNCH DETAIL

* CONCRETE HAUNCH DIMENSION MEASURED BETWEEN BOTTOM OF SLAB AND TOP OF TOP

THE MAXIMUM EMBEDMENT OF THE EDGE OF THE TOP FLANGE IN THE SLAB SHALL BE 1 INCH.

AT LEAST 21 INCHES CLEAR OF THE TOP OF THE SLAB. THESE REQUIREMENTS WERE USED IN

SHEAR STUDS ARE TO HAVE A MINIMUM PENETRATION OF 2 INCHES INTO THE SLAB AND BE

SETTING THE MAXIMUM AND MINIMUM ALLOWABLE FIELD HAUNCH VALUES SHOWN IN THE

"MISCELLANEOUS DATA TABLE" SHOWN ELSEWHERE ON THESE PLANS.

FLANGE PLATE AS SHOWN ON THE "THEORETICAL CONCRETE HAUNCH DIAGRAM" SHOWN

STRAIGHT LINE

FLSEWHERE ON THESE PLANS.

BETWEEN HAUNCHES -

DESIGN TEAM

40' RDWY. WELDED GIRDER CROSS SECTION (SYMM. CROWN) - LRFD DESIGN

3" DRIP GROOVE 2

W24×68

??? @ 50°

STANDARD SHEET 4310

ABUT.

ABUT.

SECTION A-A

(NORMAL TO ABUTMENT)

NOTE: TRANSVERSE SLAB REINFORCING

NOT SHOWN. PLACE 5d HOOPS

PARALLEL TO LONGIT. 6b1 BARS

PROJECT NUMBER

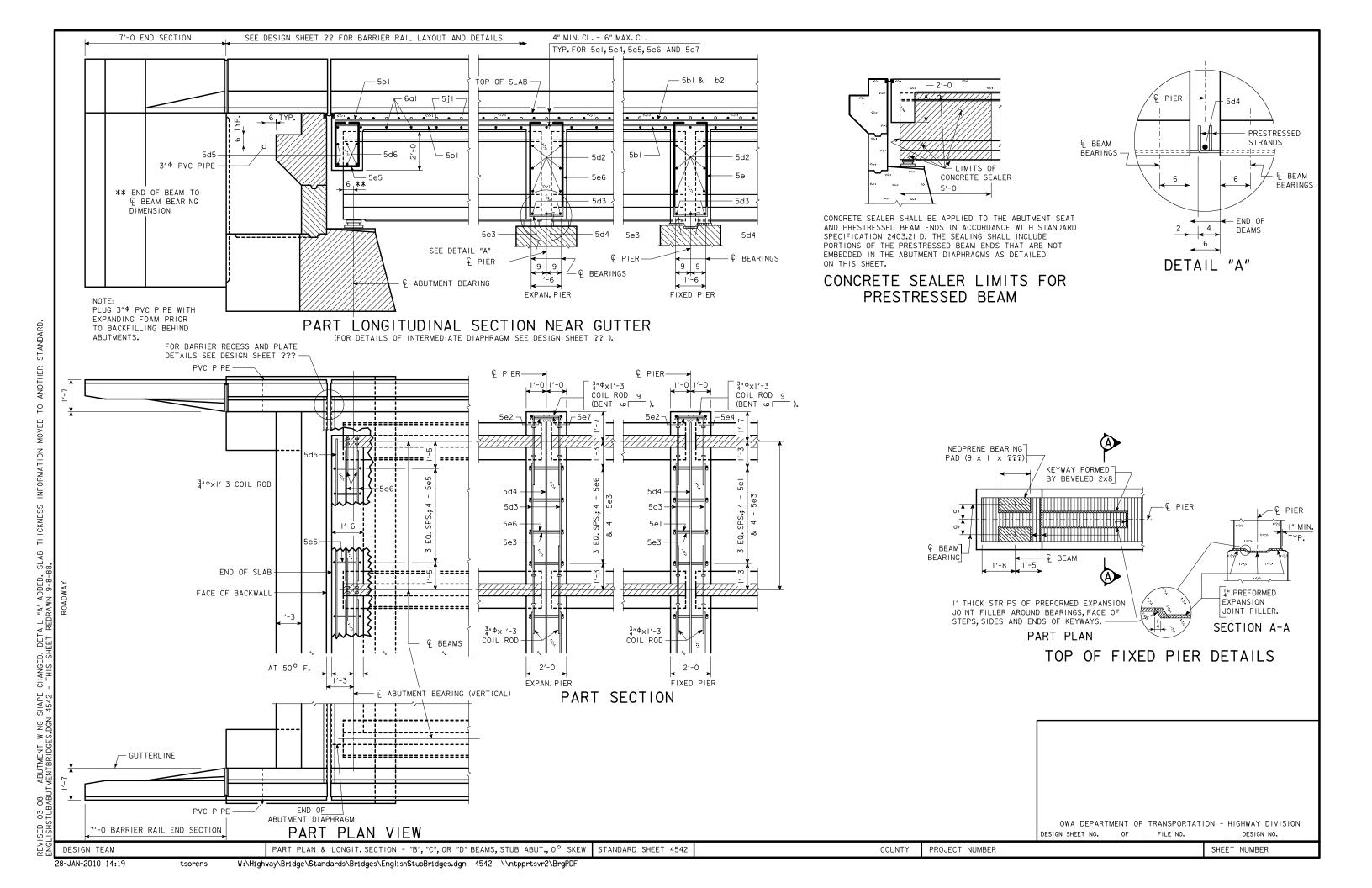
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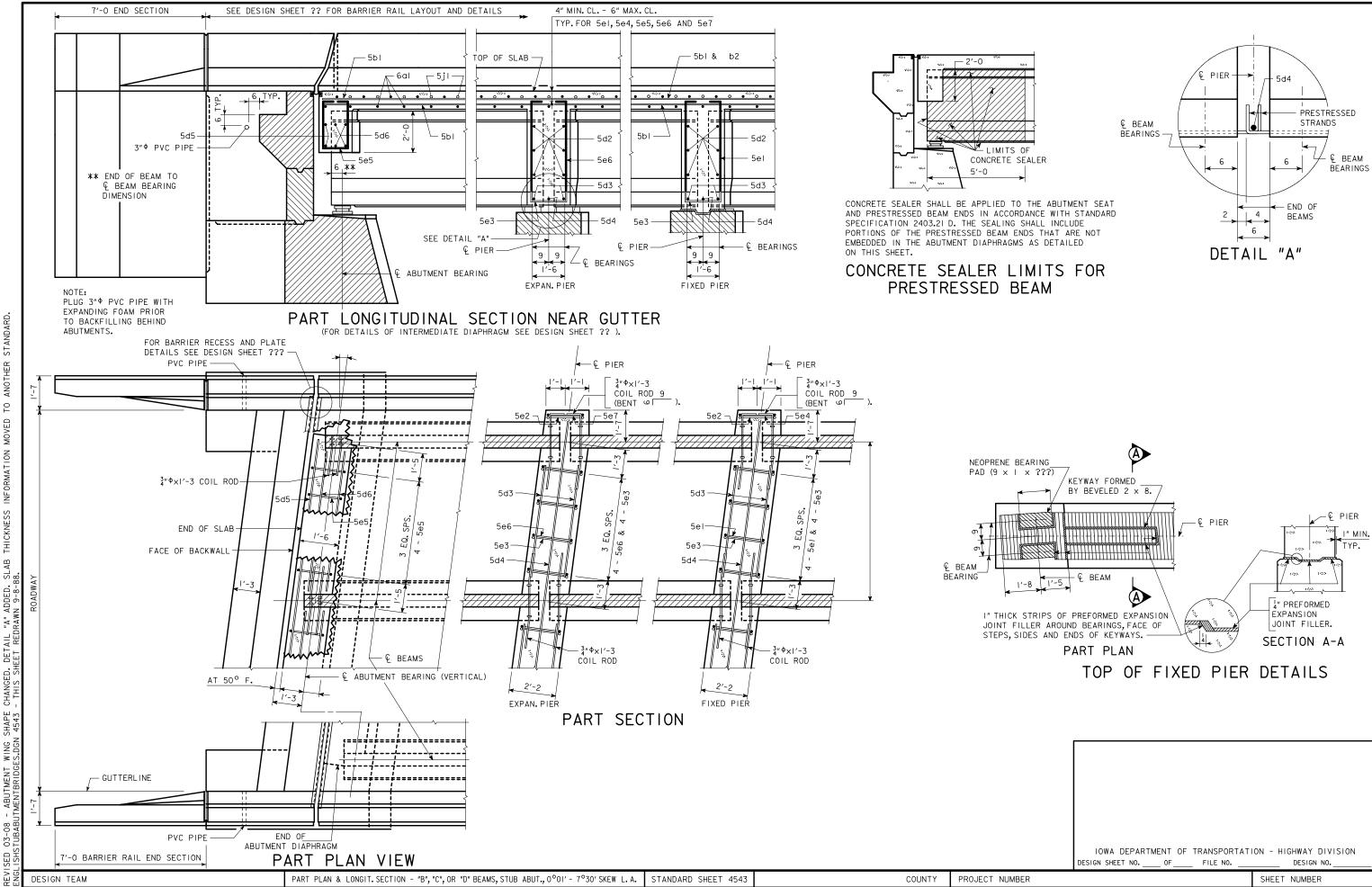
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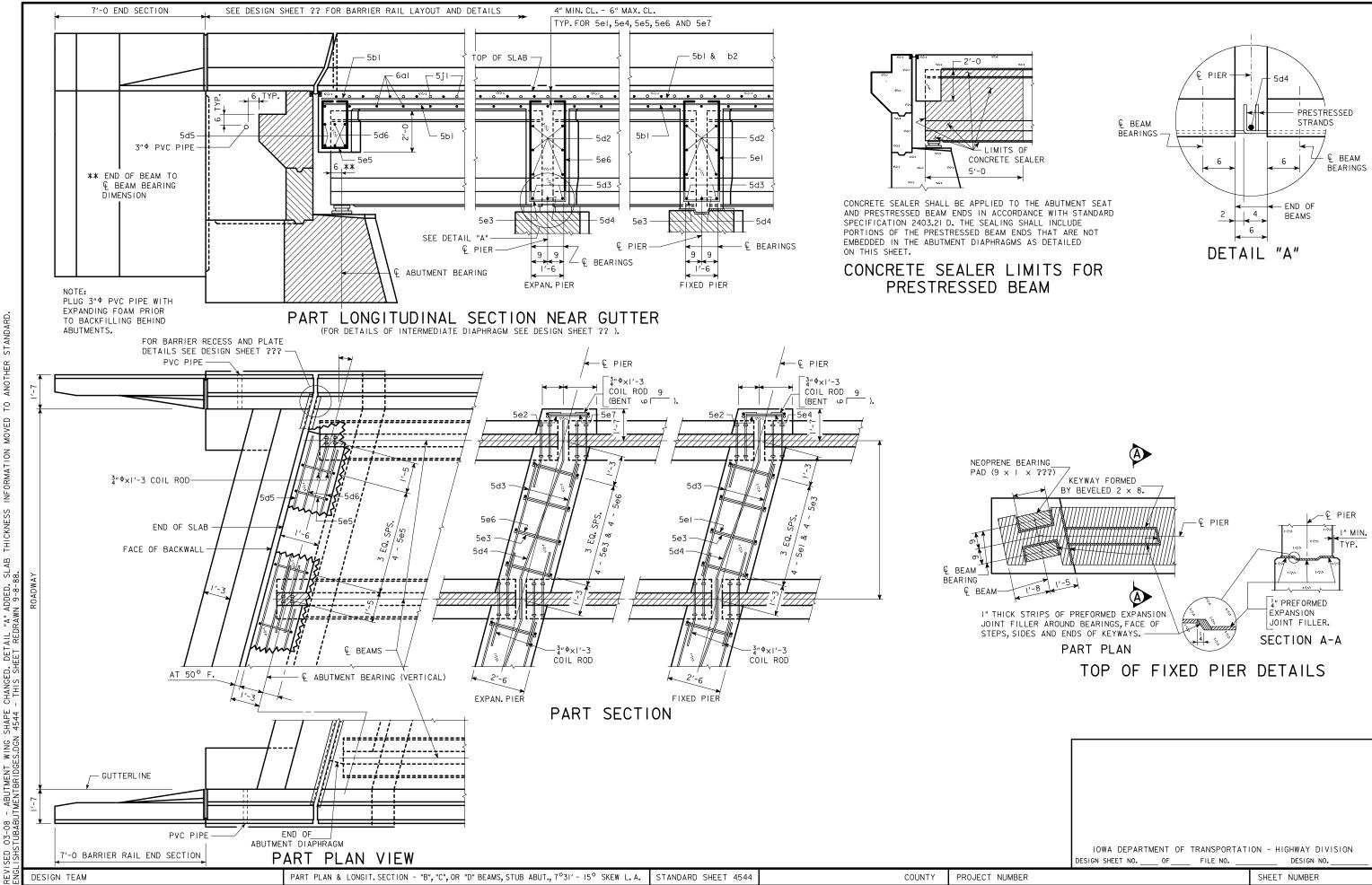
IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION

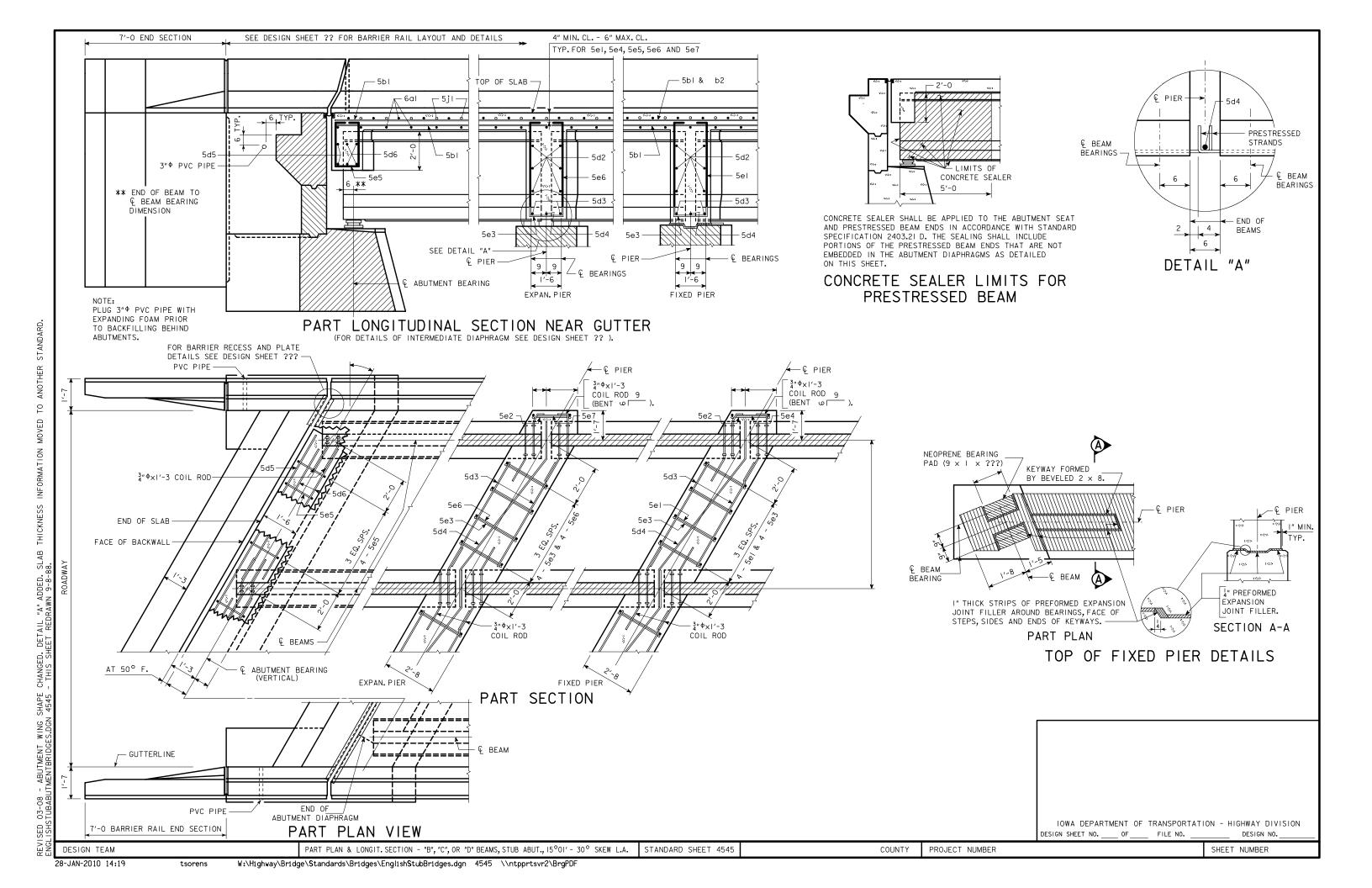
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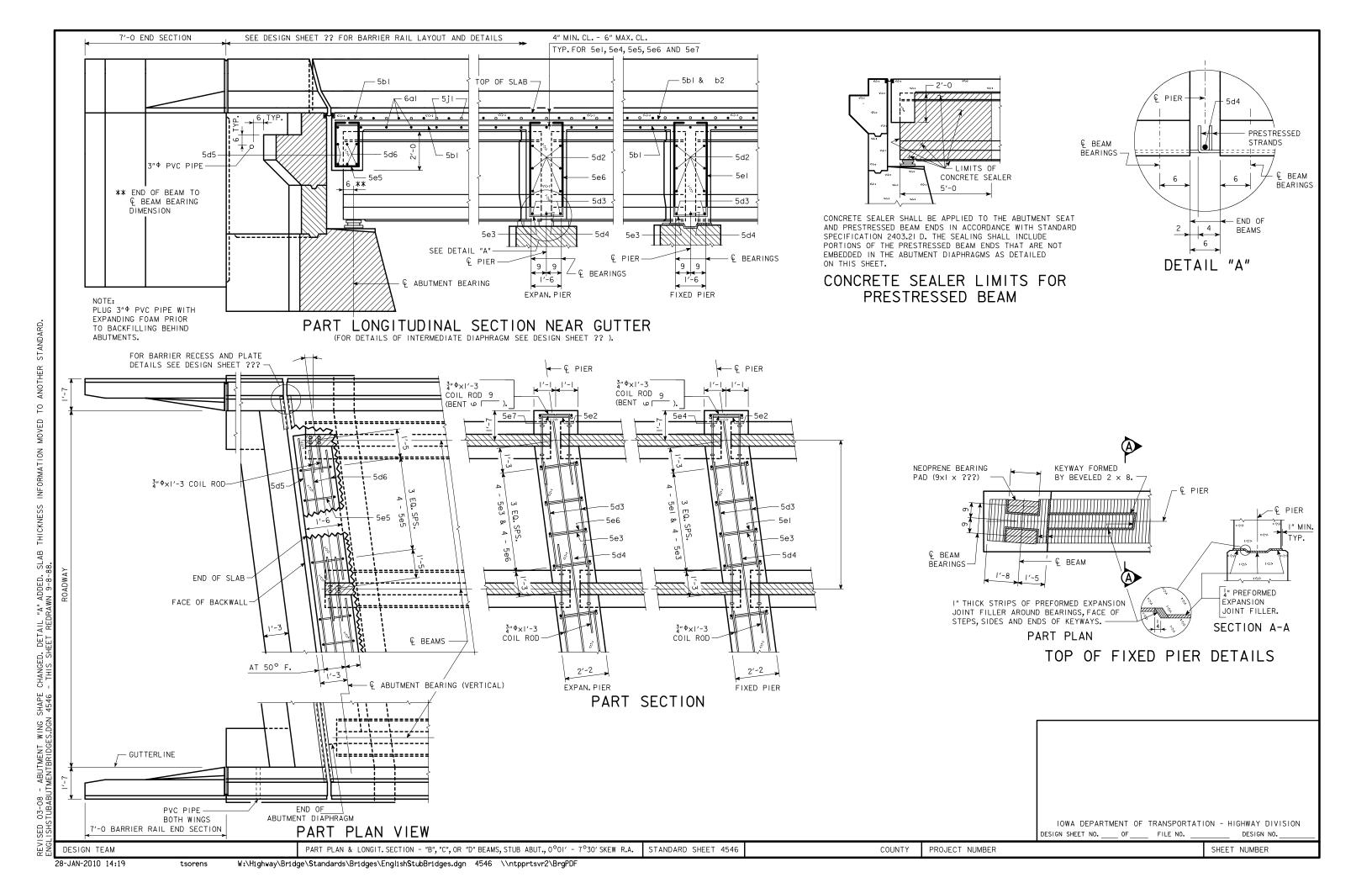
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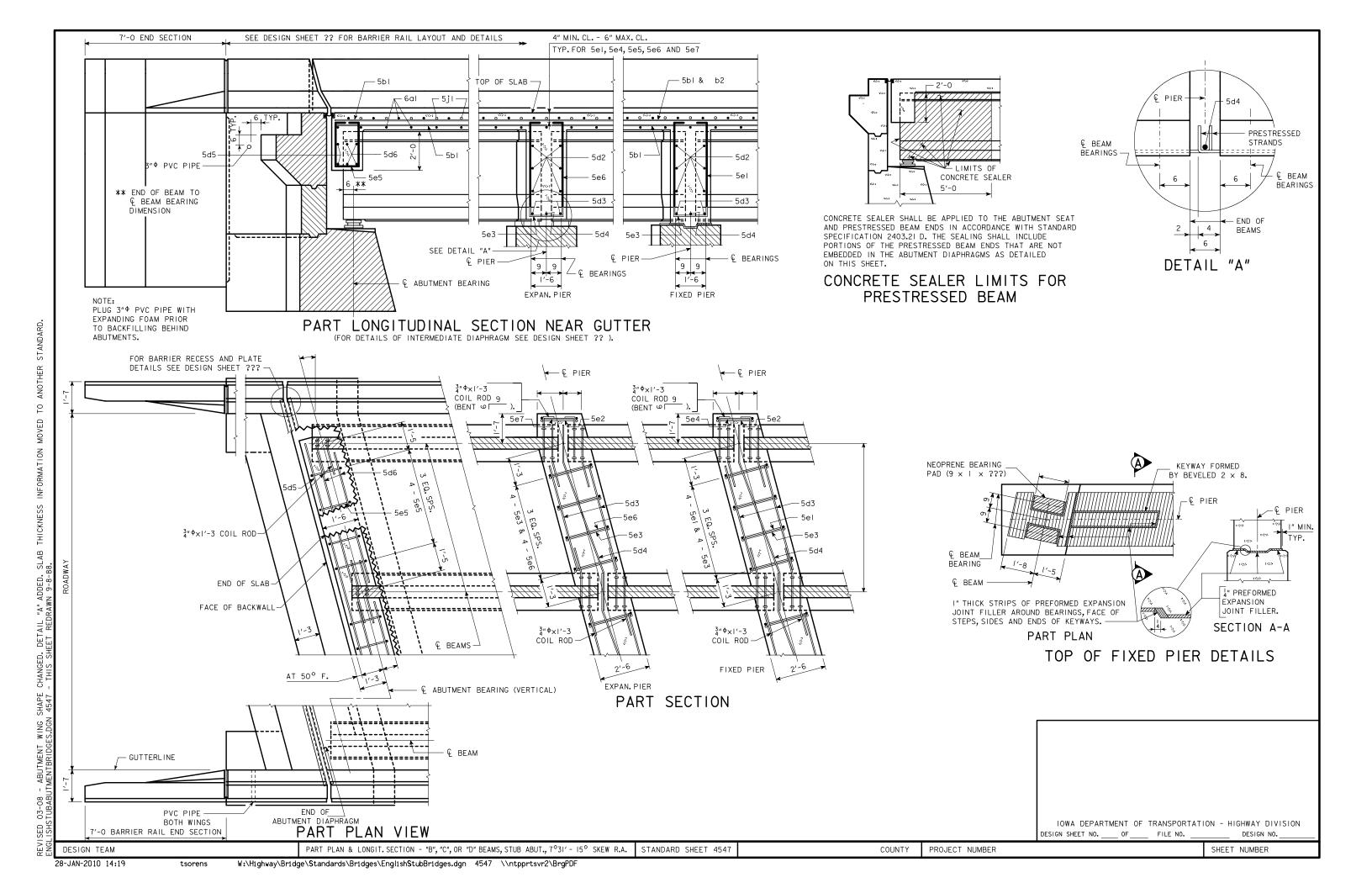


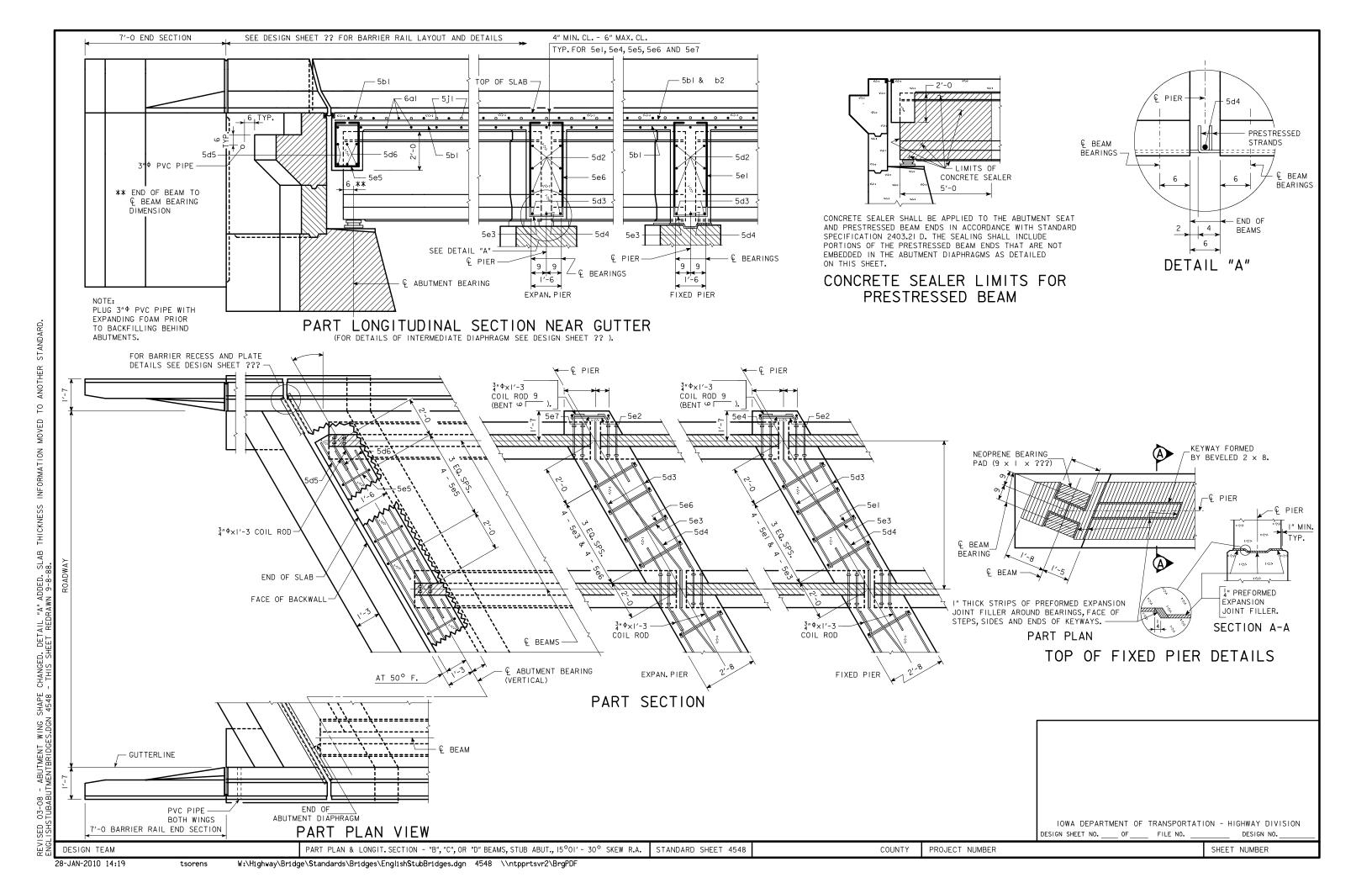


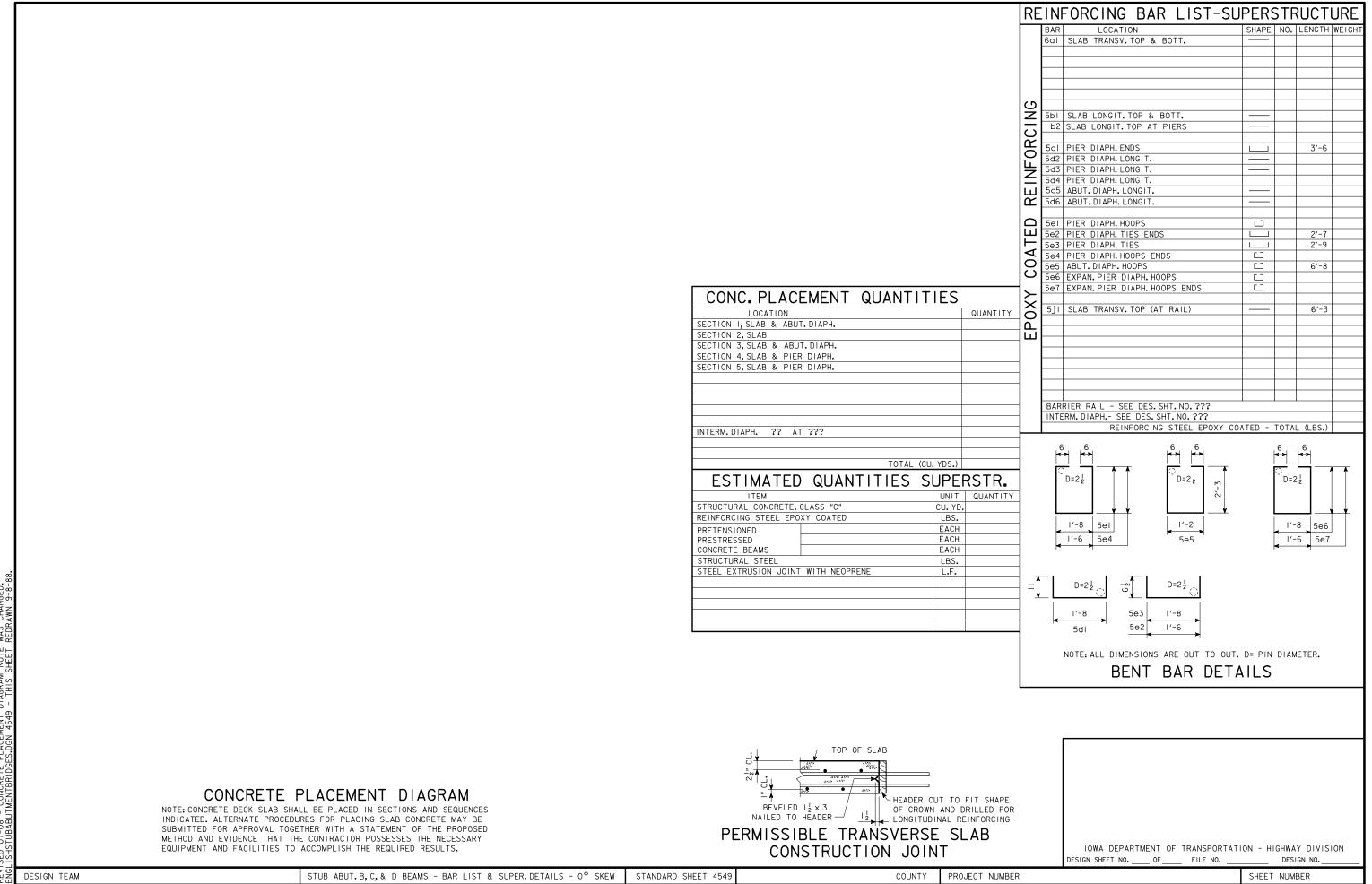


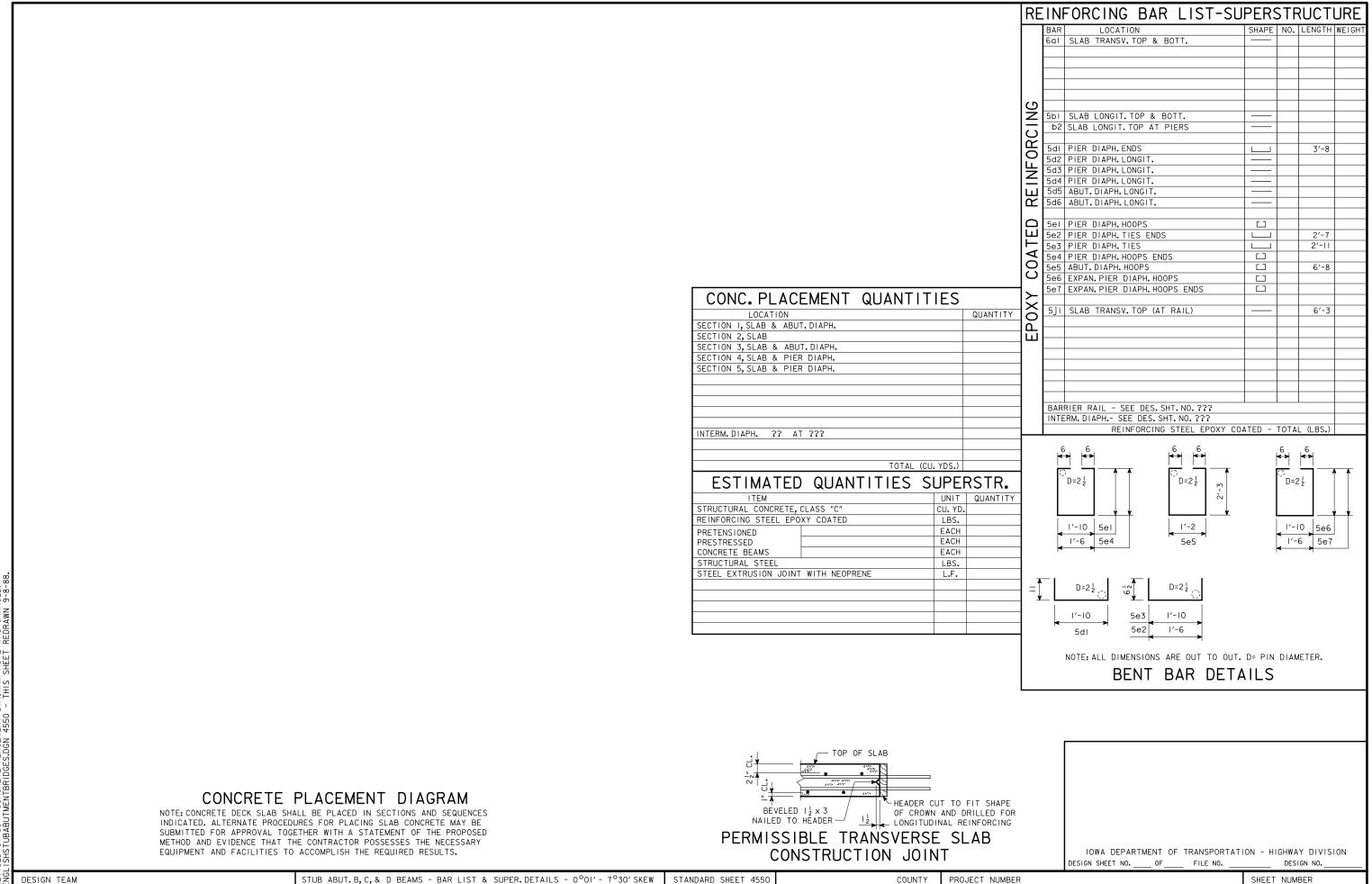


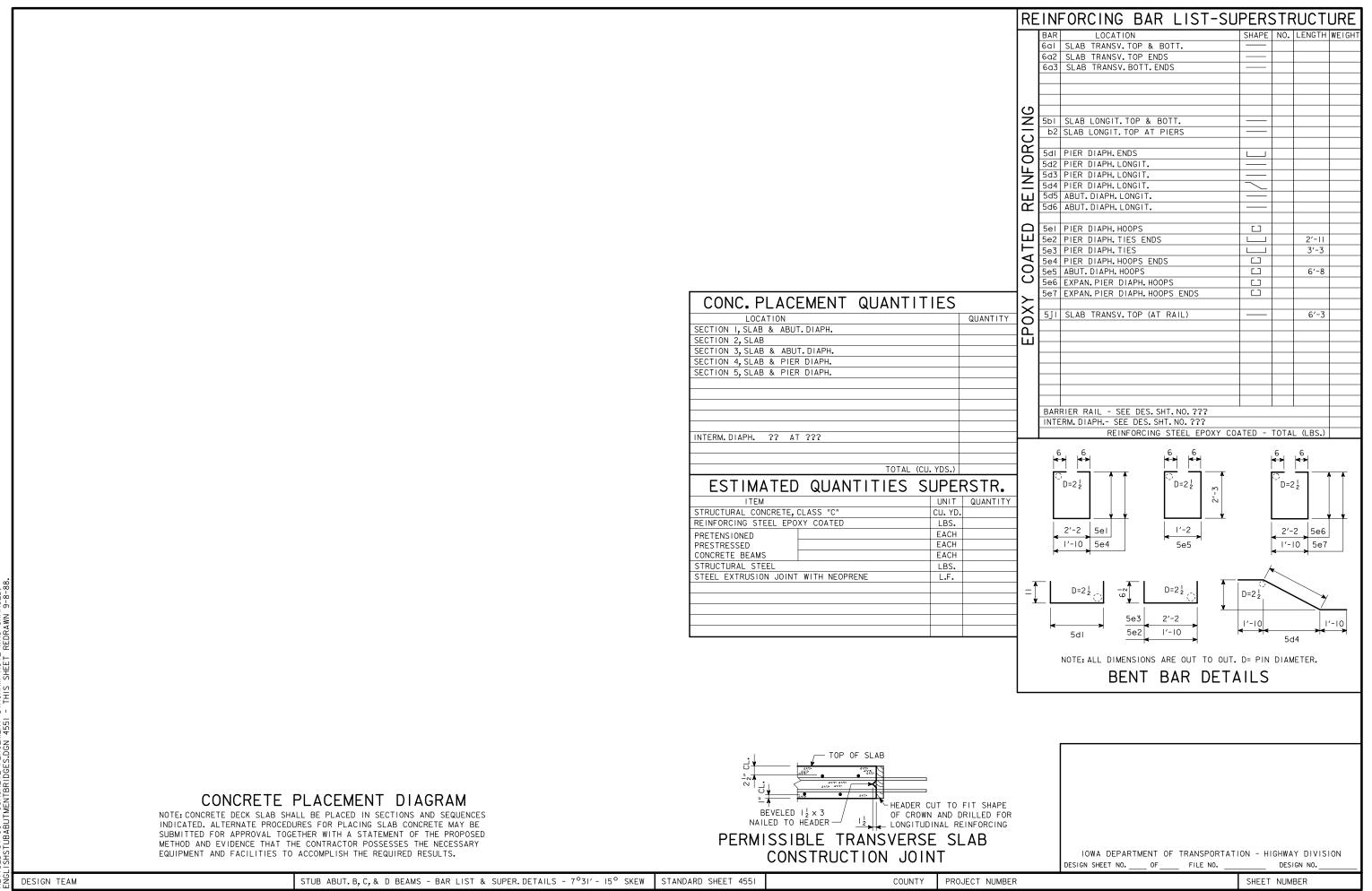




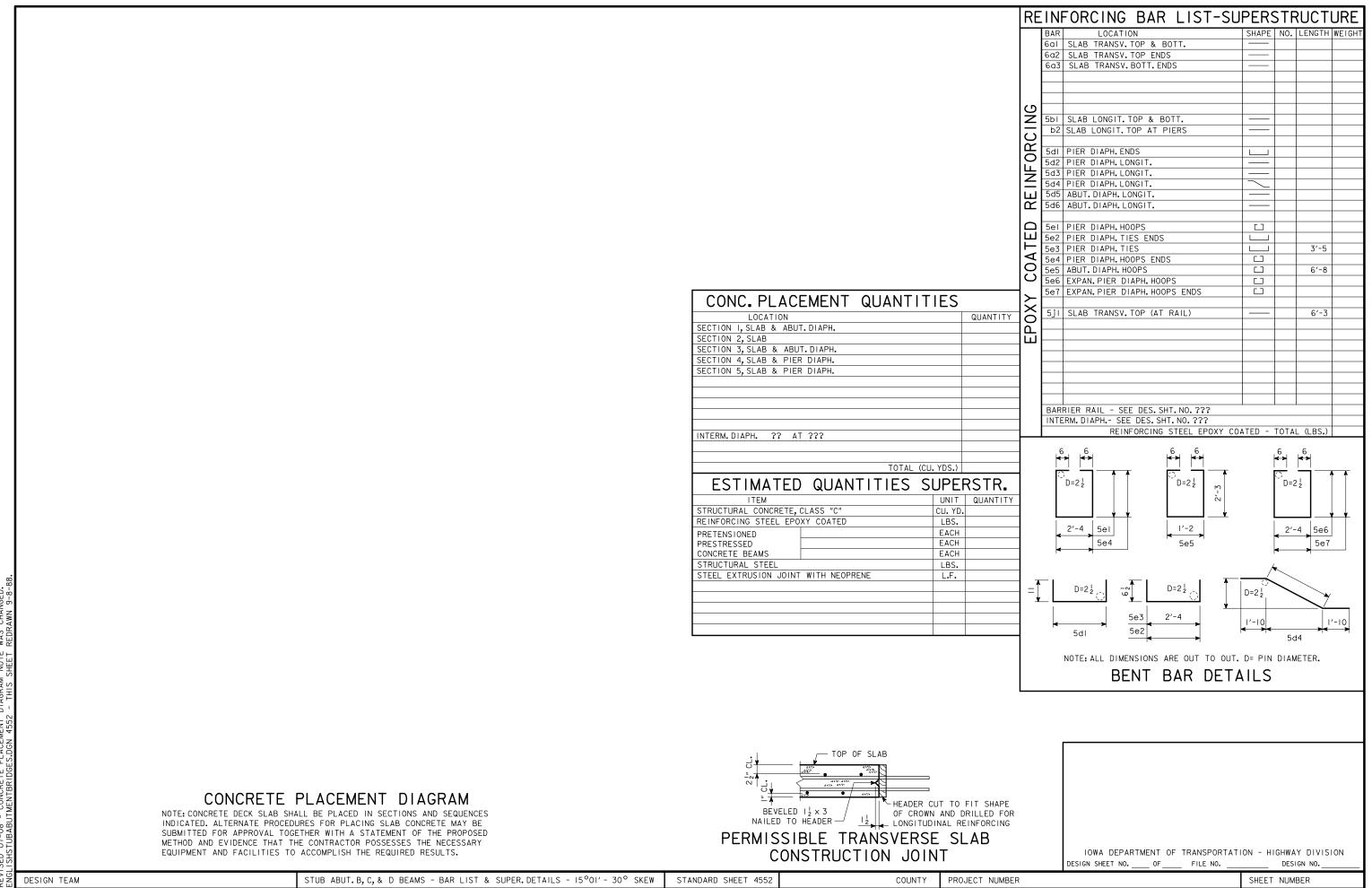


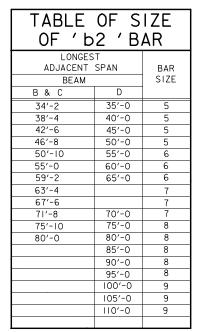




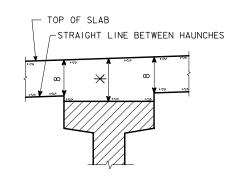


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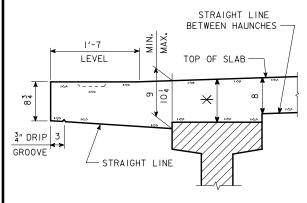




THE MIDPOINT OF THE 'b2' BAR IS TO BE PLACED AT THE & OF PIER.



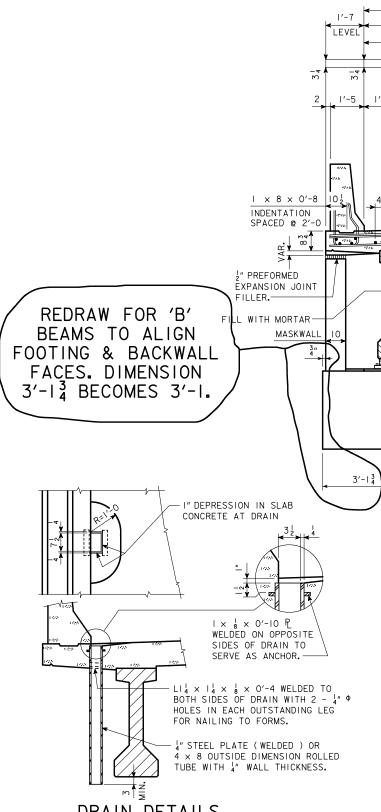
INTERIOR BEAMS



EXTERIOR BEAMS

TYPICAL SLAB AND HAUNCH DETAIL

* FOR SLAB THICKNESS OVER BEAMS SEE "SLAB THICKNESS DETAILS" ON DESIGN SHEET NO. ??.



DRAIN DETAILS

DRAINS ARE TO BE GALVANIZED. ?? DRAINS REQUIRED. SEE "SITUATION PLAN" ON DESIGN SHEET ?? FOR LOCATION. WEIGHT OF DRAINS IS INCLUDED IN THE QUANTITY FOR "STRUCTURAL STEEL". WEIGHT IS BASED ON ROLLED TUBE.

DATA F	OR C	NE D	RAIN
BEAM SIZE	В	С	D
WT. LBS.	82	92	106
LENGTH FT.	4'-23	4'-83	5′-5¾

SUPERSTRUCTURE NOTES:

HALF SECTION NEAR ABUTMENT

12'-0 TANGENT ON 2.0 % SLOPE

8 10 5 SP. @ 0'-9=3'-9 10 8

6 SP.@ 0'-9=4'-6

15 - 5bl BARS

 $\frac{3}{4}$ " $\phi \times 1' - 3$

COIL ROD

STANDARD SHEET 4556

5j1 BARS AT 10" &'s (CENTERED BETWEEN 6a

BARS IN TOP OF SLAB)

└ 5e5

THE FLOOR SLAB AS SHOWN INCLUDES 2" INTEGRAL WEARING SURFACE.

THE PIER AND ABUTMENT DIAPHRAGM CONCRETE IS TO BE PLACED MONOLITHICALLY WITH THE FLOOR SLAB.

COST OF ALL PREFORMED EXPANSION JOINT FILLER MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)". ALL BEAMS ARE TO BE SET VERTICAL.

30'-0 ROADWAY

SYMMETRICAL ABOUT

TOP OF SLAB

ROADWAY

5d2

∠5d3 & 5d4

3'-0 PARABOLIC

CROWN

5d5 & 5d6

4 BEAM SPACES @ 6'-9 = 27'-0

COUNTY

PROJECT NUMBER

TYPICAL 5bl SPACING

(TOP OF SLAB)

TYPICAL 5bl SPACING

(BOTTOM OF SLAB)

FORMS FOR THE SLAB AND BARRIER RAIL ARE TO BE SUPPORTED BY THE PRESTRESSED CONCRETE BEAMS.

CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2 INCHES UNLESS OTHERWISE NOTED OR SHOWN. ALL SLAB AND DIAPHRAGM REINFORCING IS TO BE WIRED IN PLACE

AND ADEQUATELY SUPPORTED BEFORE CONCRETE IS PLACED. TOP TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND

"CLEAR BELOW TOP OF SLAB. BOTTOM TRANSVERSE REINFORCING STEEL IS TO BE PARALLEL TO AND I" CLEAR ABOVE BOTTOM OF SLAB. TOP AND BOTTOM REINFORCING STEEL IS TO BE SUPPORTED BY INDIVIDUAL EPOXY COATED METAL BAR CHAIRS SPACED AT NOT MORE THAN 3'-0 CENTERS LONGITUDINALLY AND TRANSVERSELY, OR BY CONTINUOUS ROWS OF EPOXY COATED METAL BAR HIGH CHAIRS OR SLAB BOLSTERS SPACED 4'-0 APART.

NOTE : FOR DETAILS OF INTERMEDIATE DIAPHRAGMS SEE DESIGN SHEET ??. 477 AVE 477 - 5dl - 5e2 5d3 & 5d4 - $-\frac{3}{4}$ " $\phi \times 1'-3$ COIL $_{\Delta}^{3}$ " $\phi \times 1'-3$ ROD (BENT). COIL ROD-STRAIGHT LINE BETWEEN TOP OF FILLETS BETWEEN BEAMS.

SLAB AREA = 22.67 SQ. FT.

FOR DETAILS OF

RAIL AND RAIL

SEE STANDARD

BARRIER RAIL

5 5bl BARS

 $\frac{3}{4}$ " $\phi \times 1'-3$ COIL

ROD (BENT).

5dl

- 5e2

1'-7

SHEET. -

REINFORCING

SLAB AREA DOES NOT

INCLUDE THE NOMINAL

5el - 5e3

 $\frac{3}{4}$ " $\Phi \times 1' - 3$

COIL ROD-

HALF SECTION NEAR FIXED PIER

INCH HAUNCH.

b2 SPACING [1'-12 6 SP. @ 0'-9 = 4'-6 [1'-12

PART SECTION NEAR EXPANSION PIER

IOWA DEPARTMENT OF TRANSPORTATION - HIGHWAY DIVISION DESIGN SHEET NO. 0F FILE NO. DESIGN NO. SHEET NUMBER

30' RDWY. PPCB (B, C & D BEAMS - STUB ABUT.) CROSS SECTION - LRFD DESIGN

tsorens

DESIGN TEAM

28-JAN-2010 14:19

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